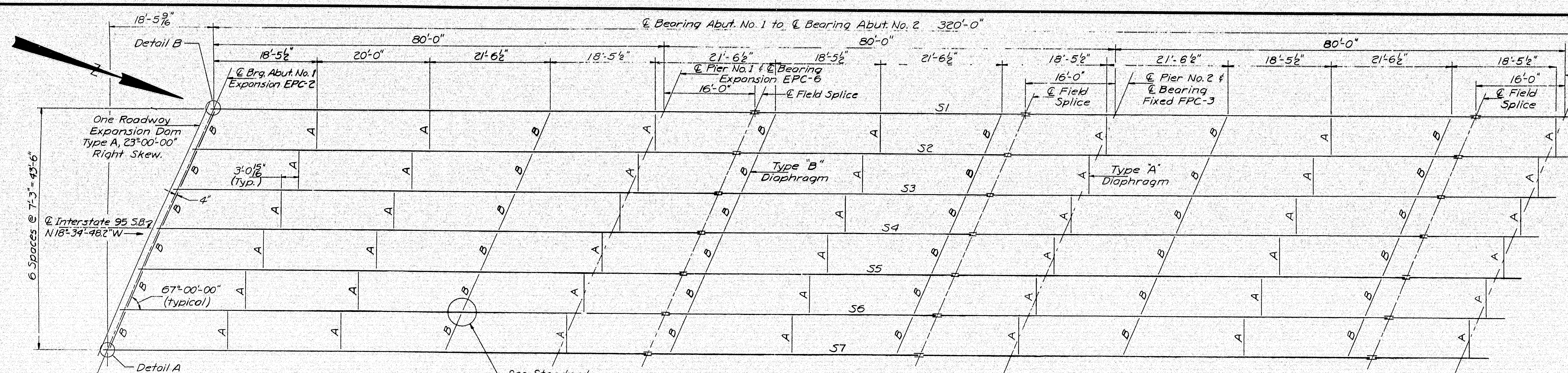


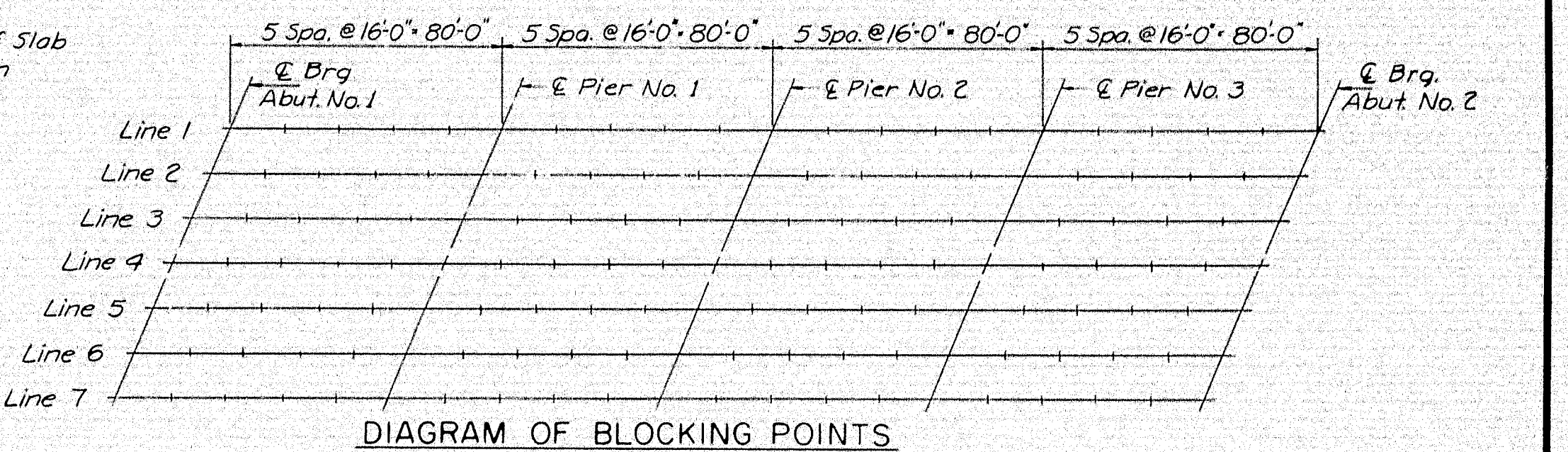
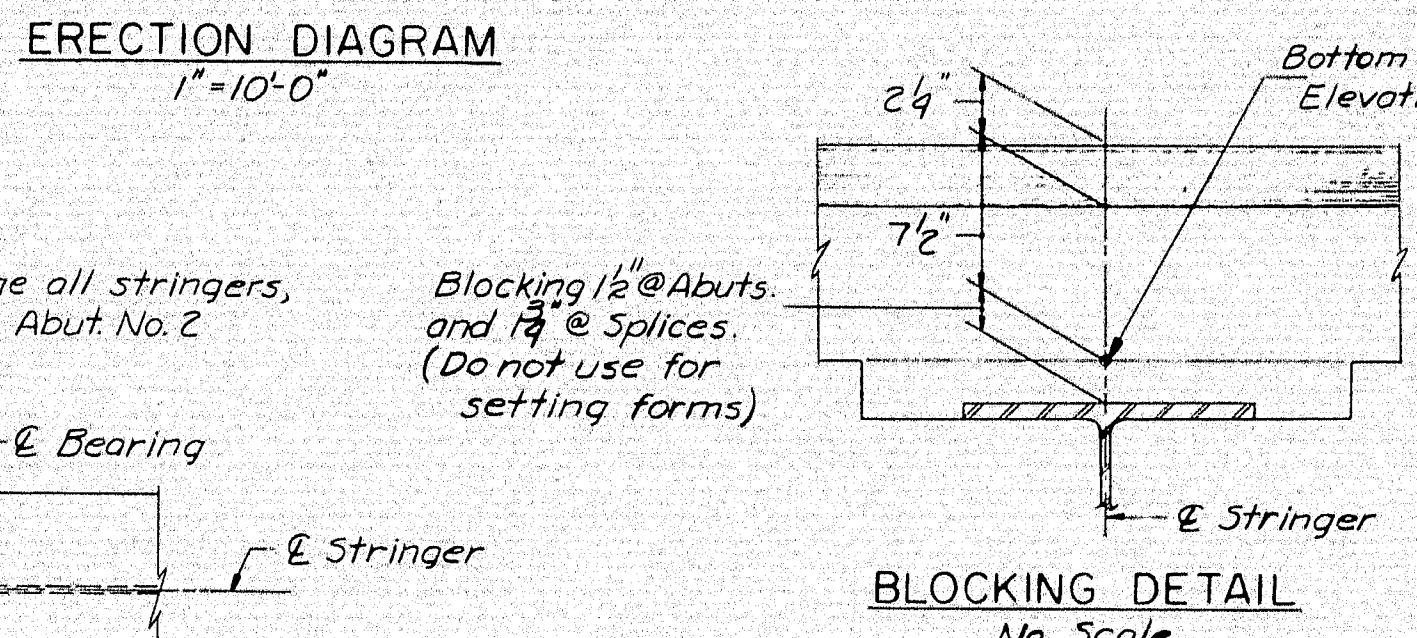
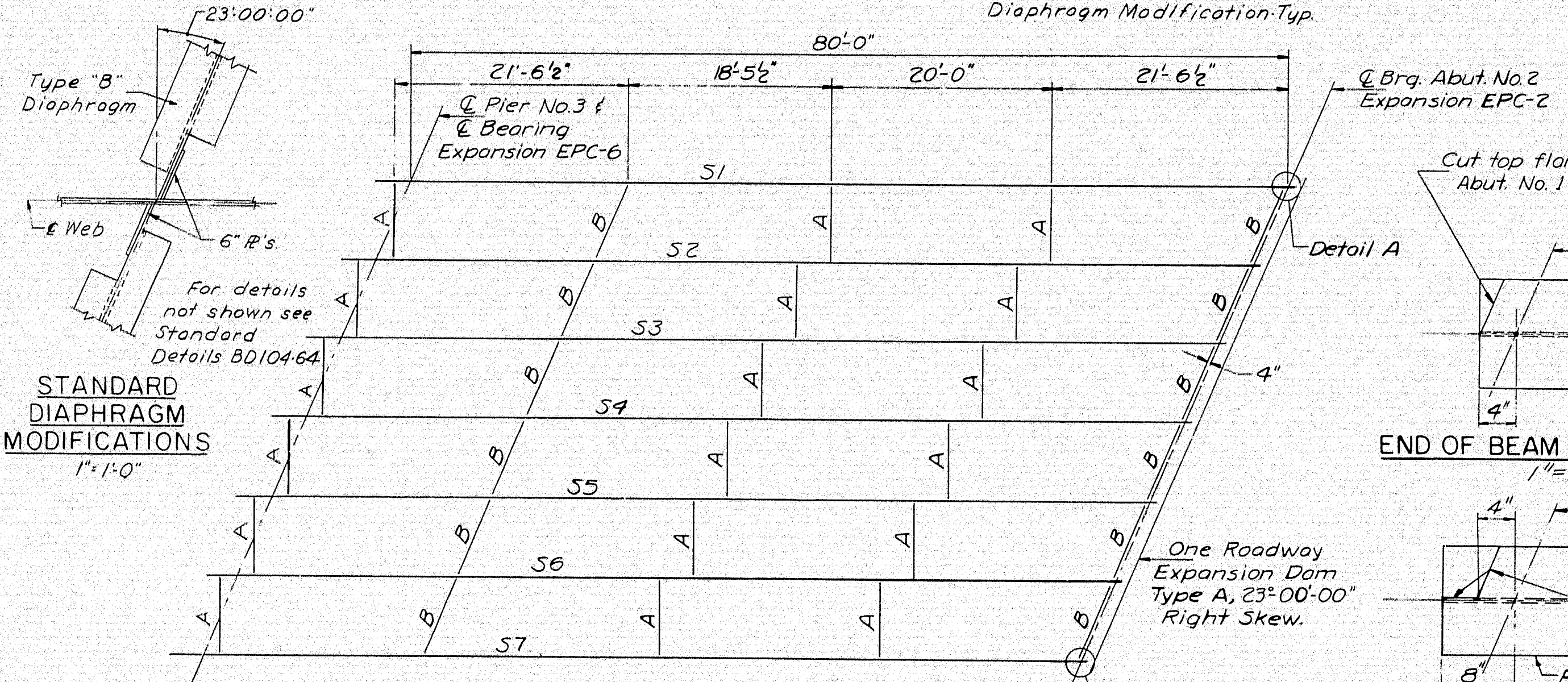
PEDESTALS
 EPC-2 14 Required
 EPC-6 14 Required
 FPC-3 7 Required

REFERENCE
 Splice - See Standard Details, BD 103-64.
 Pedestals - See Standard Details, BD 101-64.
 Expansion Dams - See Standard Details, BD 105-64.
 Diaphragms - See Standard Details, BD 104-64.
 and Standard Diaphragm Modification this sheet.
 Shear Connectors - See Standard Details BD 104-64.

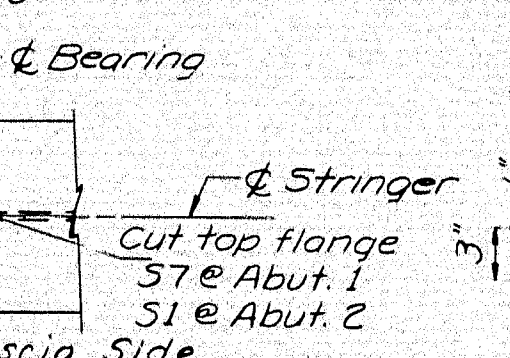
SPECIFICATION
 Fabrication and Erection: State of Maine
 Standard Specifications, Highways and Bridges,
 Revision of Jan. 1956 & Supplemental
 Specifications of Feb. 1960.
 Design and Detail: A.A.S.H.O. Standard
 Specification For Highway Bridges of 1961 and
 Interim Specifications of 1961, 1962, 1963, and 1964.
 Materials: Except as otherwise noted on the
 standard details, all material shall conform to
 A.S.T.M. designation A36.



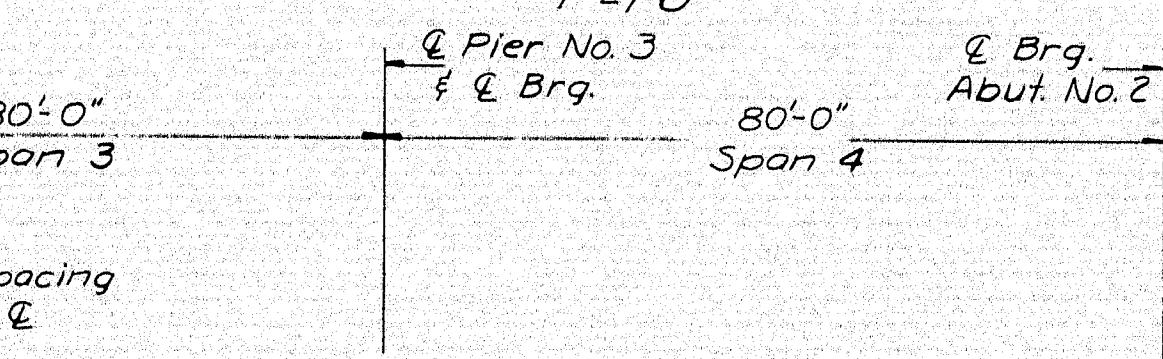
ERECTION DIAGRAM
 1"=10'-0"



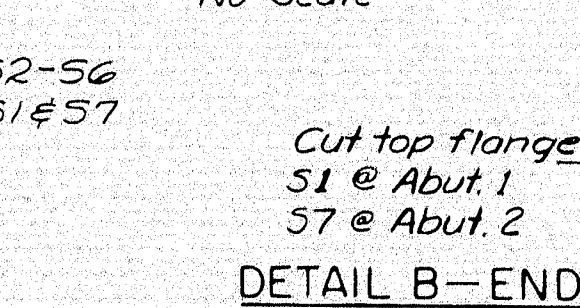
END OF BEAM DETAIL S2-S6
 1"=10'-0"



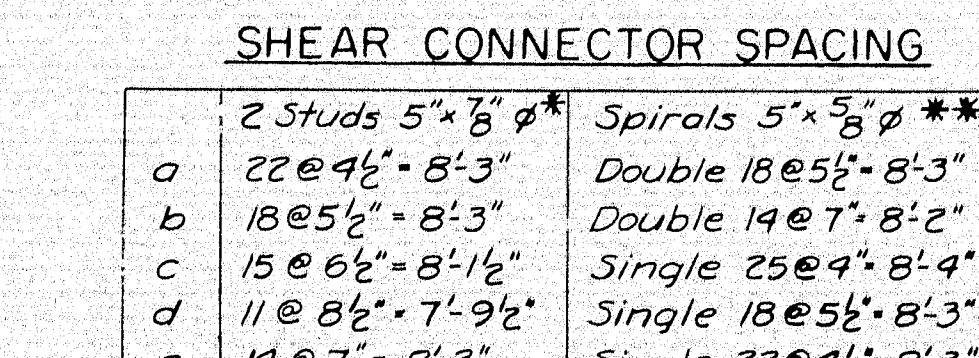
DETAIL A - END OF BEAM DETAIL S1 & S7
 1"=10'-0"



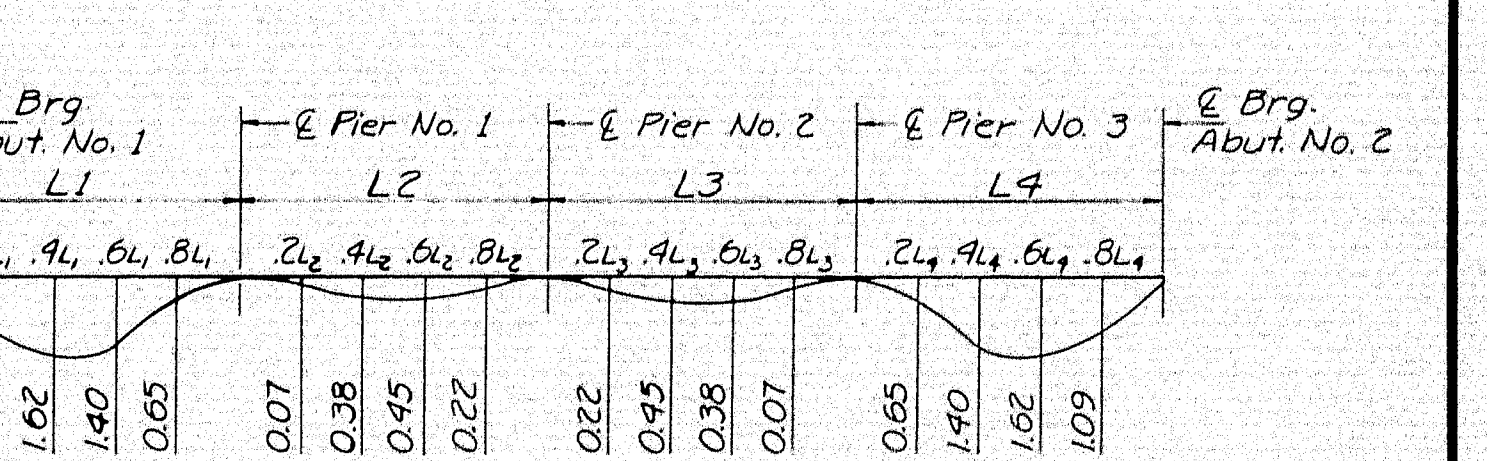
COVER PLATE DETAIL
 No Scale



DETAIL B - END OF BEAM DETAIL S1 & S7
 1"=10'-0"



BEAM GRADES



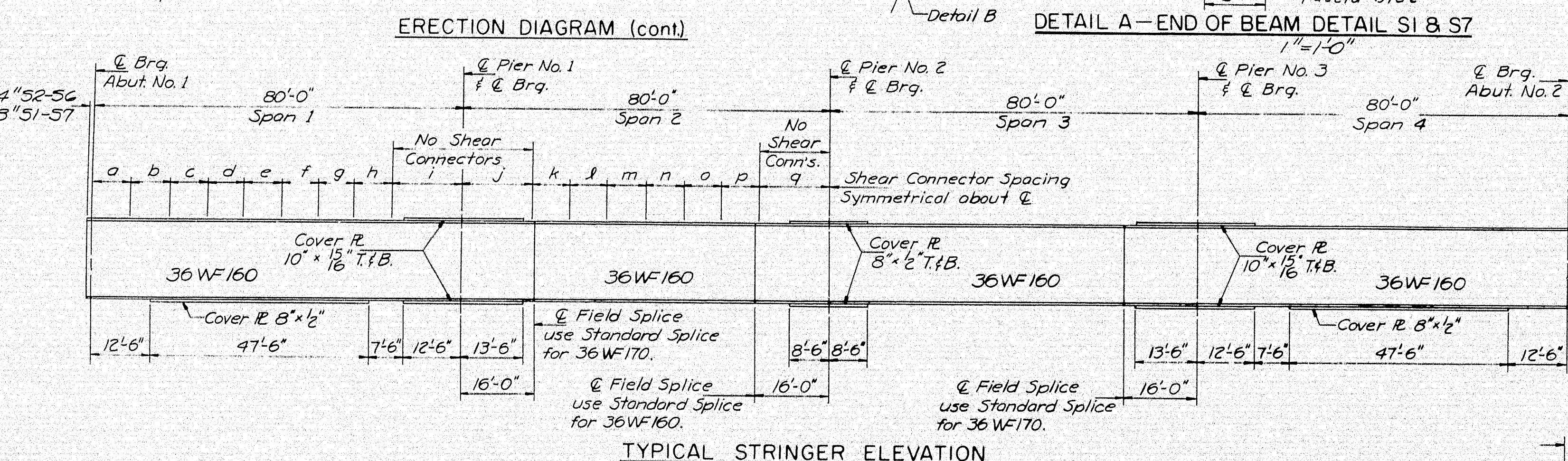
DEAD LOAD DEFLECTION DIAGRAM
 ALL DEFLECTIONS IN INCHES
 No shop camber required
 Place natural mill camber up.

NOTE
 To compensate for dead load deflections as well as possible irregularities in beams set the bottom of slab elevations at the points indicated before any of the slab formwork is started. Bottom of slab grades for blocking shall be set after Shear Connectors are welded to the top flanges.

Total number of studs 6,384

HOWARD, NEEDLES, TAMMEN & BERGENDOFF
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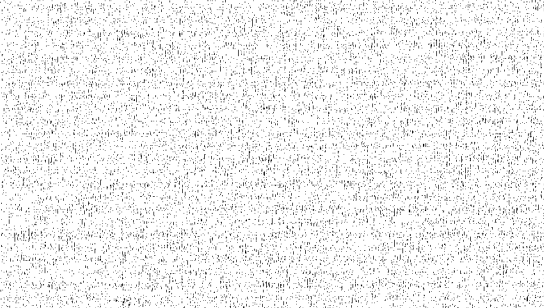


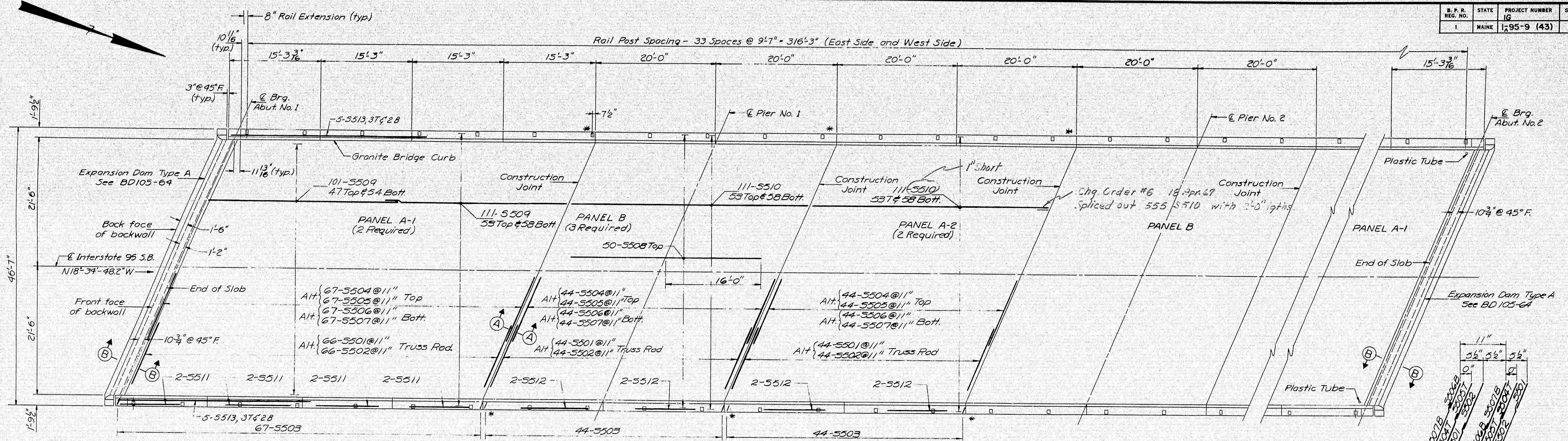
TYPICAL STRINGER ELEVATION

BOTTOM OF SLAB ELEVATIONS AT BLOCKING POINTS

Line	Abut. No. 1	Span 1	Pier No. 1	Span 2	Pier No. 2	Span 3	Pier No. 3	Span 4	Abut. No. 2
Line 1	480.75	480.67	480.54	480.37	480.15	479.95	479.79	479.65	479.49
Line 2	480.91	480.83	480.70	480.52	480.31	480.10	479.95	479.81	479.65
Line 3	481.07	480.98	480.86	480.68	480.47	480.26	480.10	479.96	479.81
Line 4	481.22	481.14	481.01	480.84	480.62	480.42	480.26	480.12	479.96
Line 5	481.37	481.29	481.16	480.99	480.77	480.55	480.32	480.16	480.03
Line 6	481.52	481.44	481.31	481.14	480.92	480.70	480.47	480.29	480.13
Line 7	481.67	481.59	481.46	481.29	481.07	480.84	480.62	480.40	480.23

SPIRAL LAP DETAIL

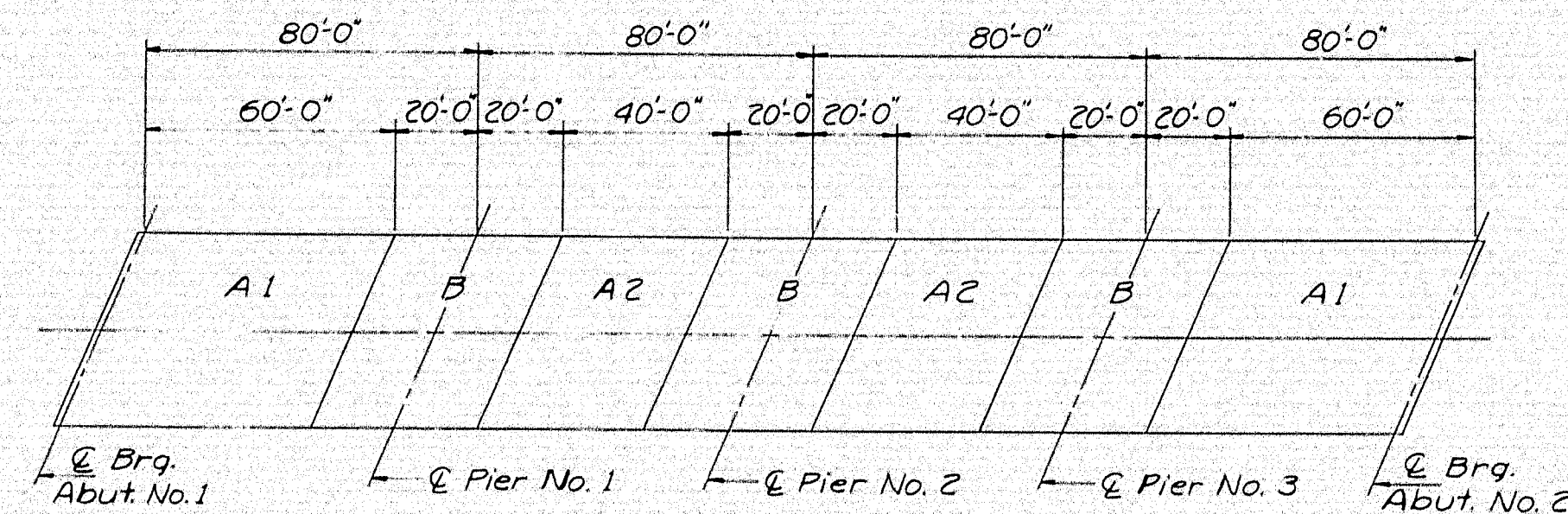




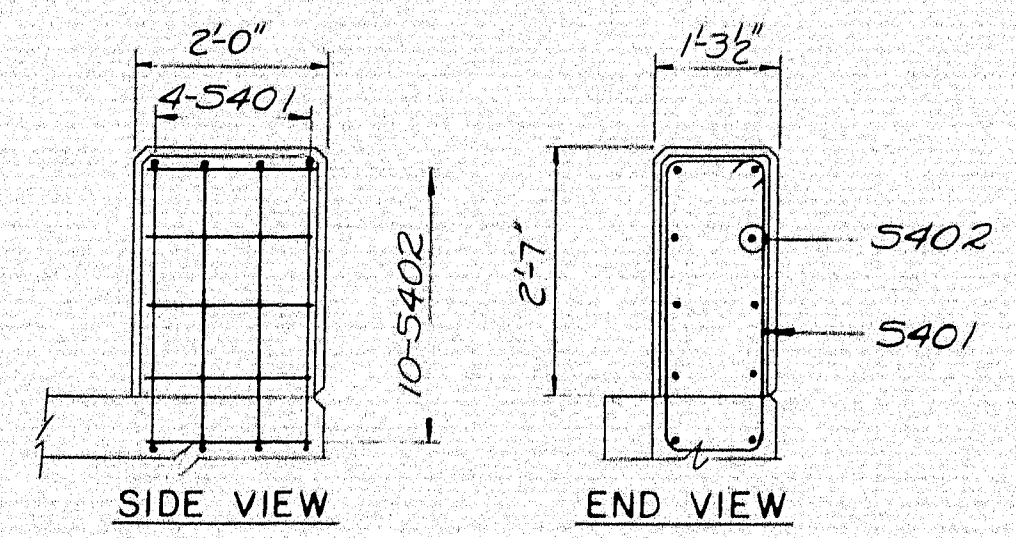
TYPICAL REINFORCING PATTERN

GENERAL SUPERSTRUCTURE NOTES

- At joints in curbs and granite bridge curbs over piers, use $\frac{1}{4}$ " preformed expansion joint filler. At all other curb joints, break the bond between concrete surfaces with a suitable grade of asphalt paint. Form V groove on outside face of curb and slab at each vertical joint. Provide joints in granite bridge curb at curb construction joints.
- At low points in slabs, place a plastic tube $1\frac{1}{2}$ " ϕ through the slab for drainage. Exact location to be determined in the field. Do not cover the tube with waterproofing. This work will be incidental to contract items. Tubes shall extend 2' below bottom of slab. Place tubes to drip clear of bridge seat.
- For bridge rail, see Standard Details, BD107-64 & BD108-64.
- Place concrete in 'A' panels before placing concrete in 'B' panels.
- Payment for concrete end posts shall be made under Item 701-40.
- Granite Bridge Curb means Vertical Bridge Curb-Type 1 and will be paid for under Items 901-24 and 901-25.
- * Field bend rod to accommodate joint in slab.
- Safety walk reinforcing applies to both sides.
- All reinforcing to have 2" minimum cover unless otherwise shown.
- No work other than form work utilizing hand tools will be permitted on the slab for a period of seven days following placing of slab.



POURING SEQUENCE



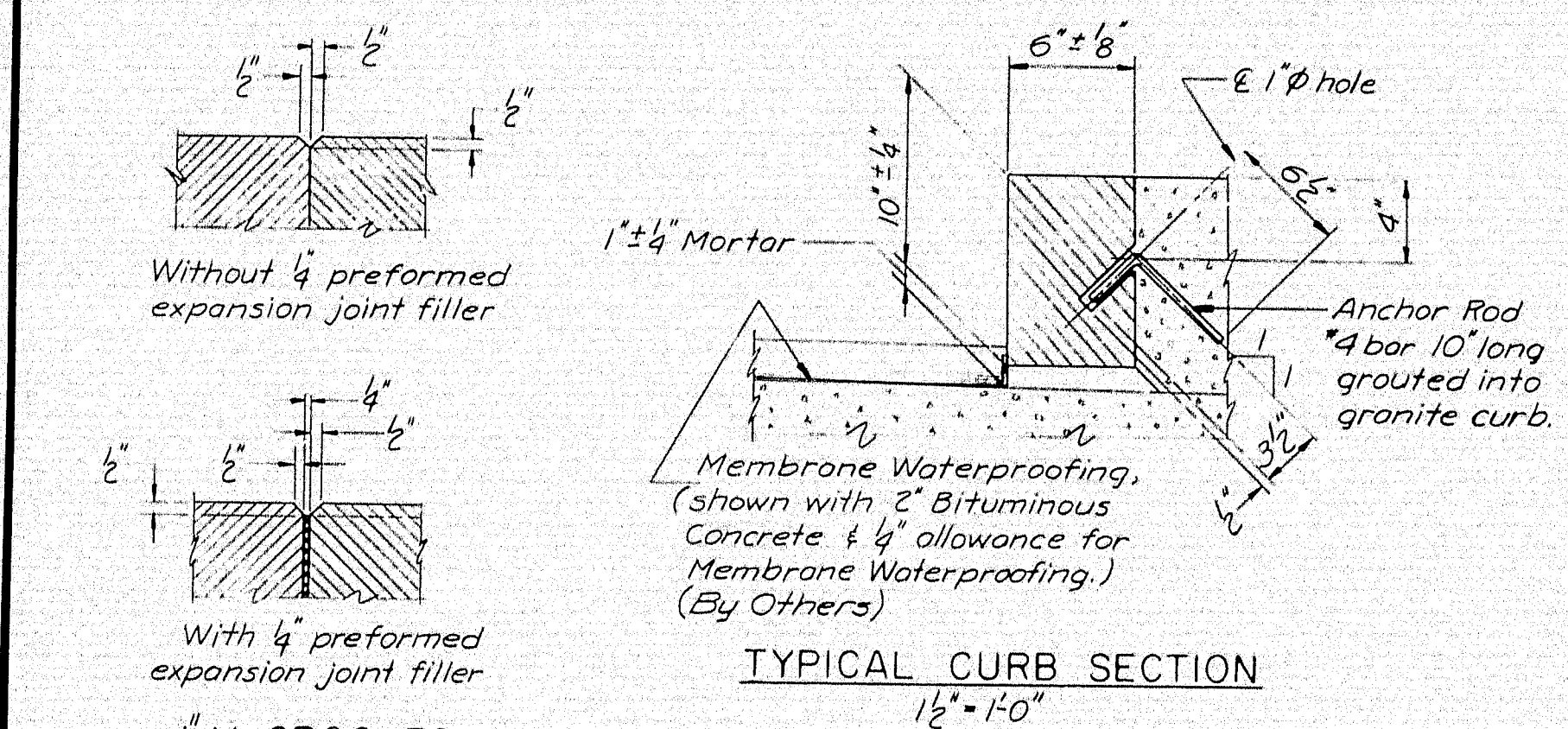
END POST DETAIL
 $\frac{1}{2}$ " = 1'-0"

BRIDGE DRAIN NOTES

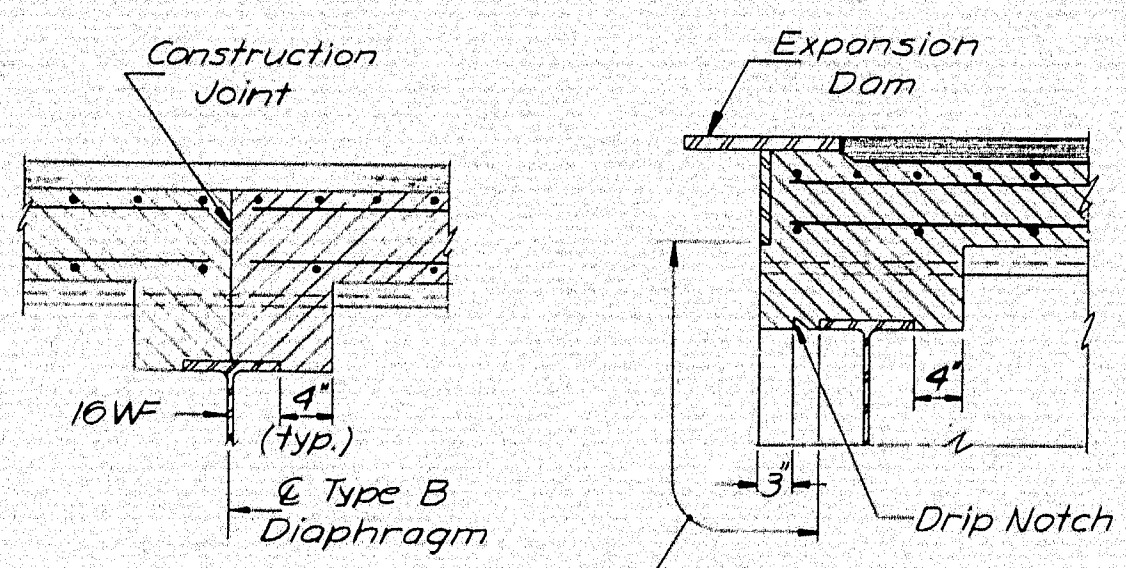
- Four drains each side, spans 1, 2, & 3. Two drains each side, span 4.
- For approximate locations see sheet 1. Exact position to be determined in field.
- Bridge drains to be placed a minimum of 10' clear of piers.

DESIGN - E.F.K. TRACE - R.R.N.	DETAIL - R.R.S.	BRIDGE NO. SURVEY - PLOT
STATE HIGHWAY COMMISSION BRIDGE DIVISION		
INTERSTATE 95 S.B. OVER		
FISH STREAM & RELOC. ROUTE SA. NO. 1		
IN THE TOWN OF ISLAND FALLS ARROSTOOK COUNTY SUPERSTRUCTURE		
SHEET 10 OF 12 AUGUSTA, MAINE AUGUST 1965		
ISLAND FALLS (43)		

101-212

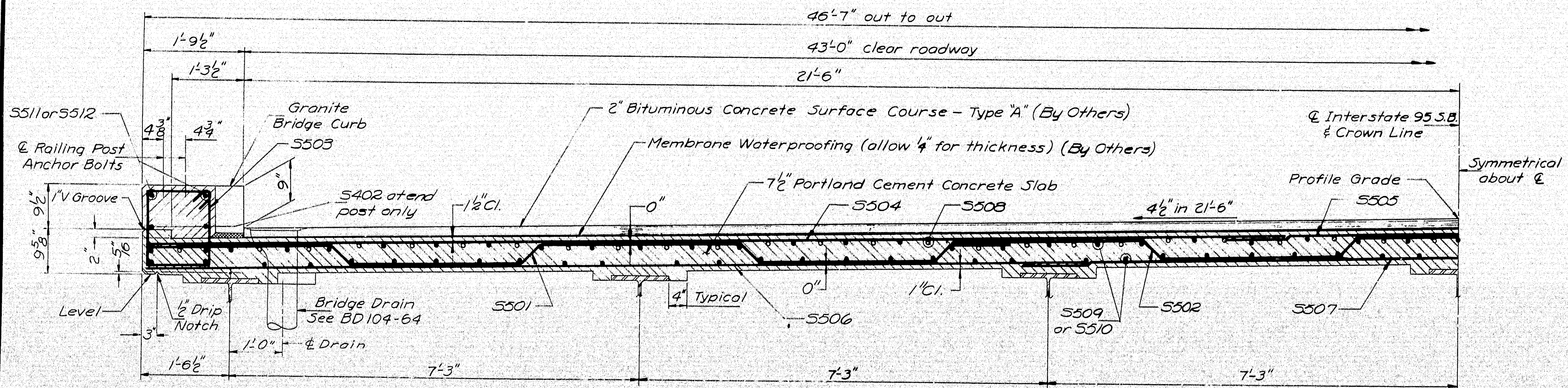


TYPICAL CURB SECTION
 $\frac{1}{2}$ " = 1'-0"

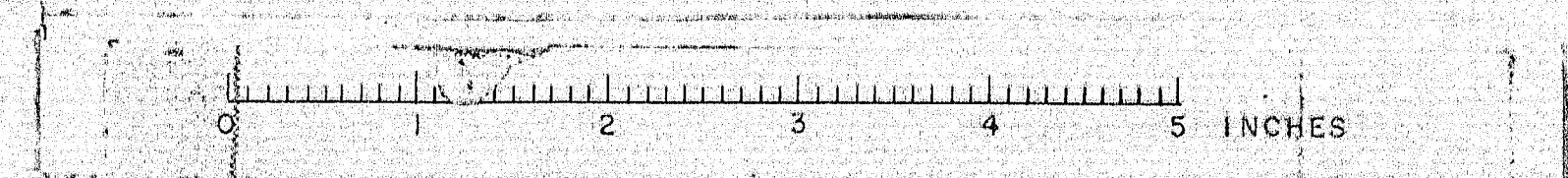


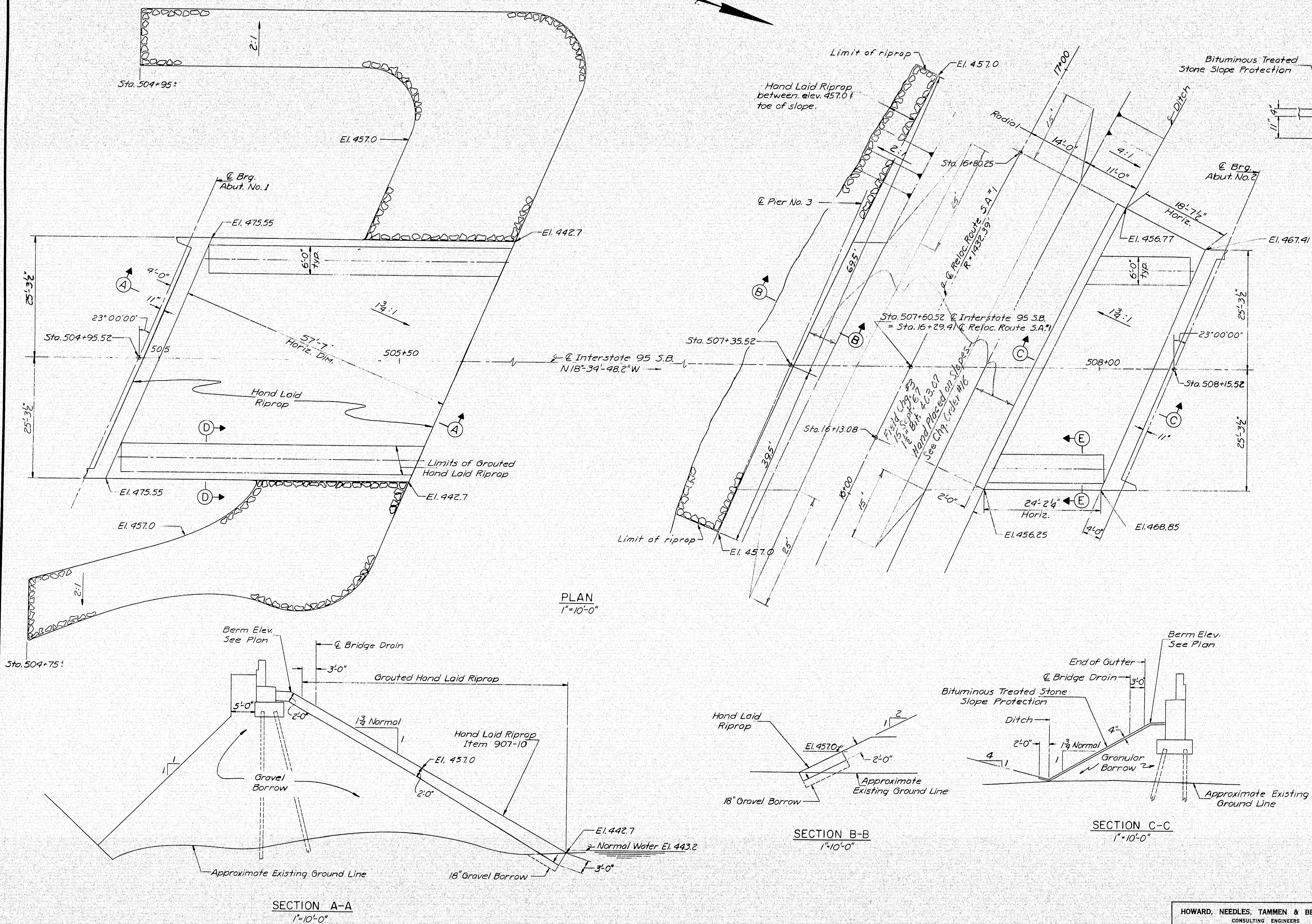
SECTION A-A
 $\frac{3}{4}$ " = 1'-0"

SECTION B-B
 $\frac{3}{4}$ " = 1'-0"



HALF TRANSVERSE SECTION
 $\frac{3}{4}$ " = 1'-0"





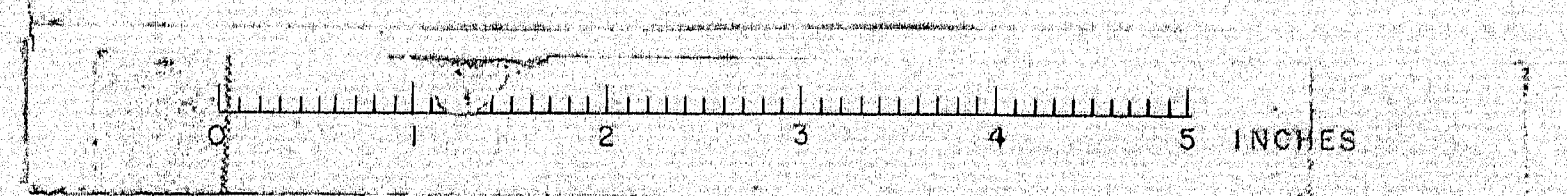
DESIGN- TRACE- CHECK-P.R.N.	DETAIL-P.R.S.	BRIDGE NO. SURVEY- PLOT-
STATE HIGHWAY COMMISSION BRIDGE DIVISION		
INTERSTATE 95 S.B.		
OVER		
FISH STREAM & RELOC. ROUTE S.A. NO. 1		
IN THE TOWN OF		
ISLAND FALLS		
AROOSTOOK COUNTY		
SLOPE PROTECTION		

HOWARD, NEEDLES, TAMMEN & BERGENDOFF
CONSULTING ENGINEERS
NEW YORK BOSTON KANSAS CITY

SHEET 11 OF 12 AUGUSTA, MAINE AUGUST 1965

101-213

ISLAND FALLS (43)

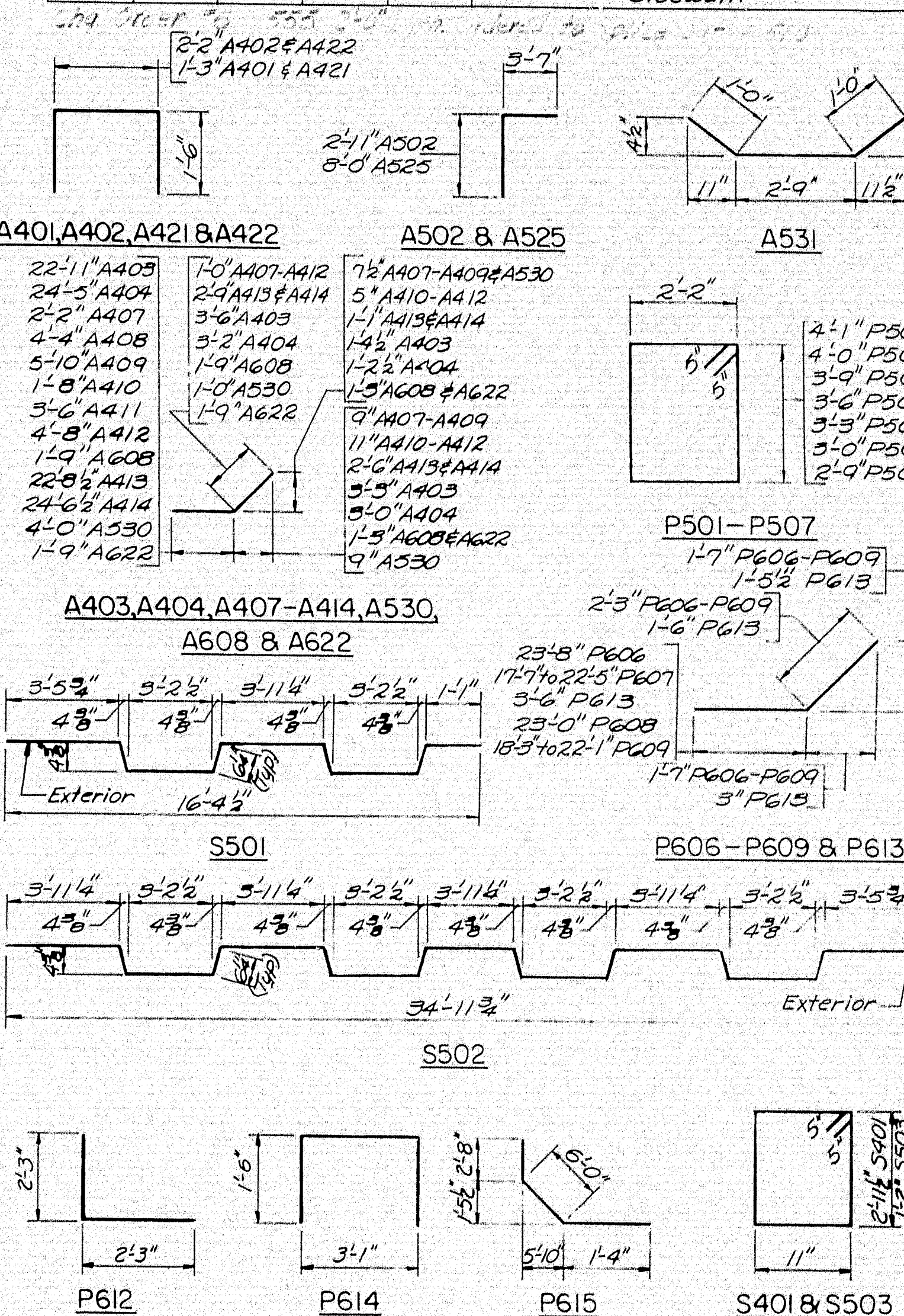


MARK	SIZE	NUMBER	LENGTH	INCR.	LOCATION
ABUTMENT 1					
STRAIGHT BARS					
A405	4	2	6'10"		Bend in field Wingwall
A406	4	2	6'10"		Bend in field "
A415	4	1	2'7"		"
A416	4	1	4'9"		"
A417	4	5	6'6"		"
A418	4	1	2'6"		"
A419	4	1	4'4"		"
A420	4	5	5'10"		Wingwall
A501	5	34	2'6"		Dowels
A503	5	34	6'3"		Stem
A504	5	8	2'10"		"
A505	5	8	2'16"		Stem
A506	5	34	3'0"		Dowels
A507	5	14	4'10"		Backwall
A508	5	10	3'6" to		
A509	5	24	5'4"	5'2"	Wingwall 2 Groups of 5
A510	5	8	2'10" to		"
			5'4"	10"	Wingwall 2 Groups of 4
A601	6	12	30'0"		Footing
A602	6	12	22'7" to		
			33'0"	2'1"	" 2 Groups of 6
A603	6	96	5'6"		"
A604	6	20	8'0"		"
A605	6	6	3'6"		"
A606	6	12	3'6" to		" 2 Groups of 6
			5'6"	4'3"	"
A607	6	16	3'6" to		"
			5'10"	4"	Footing 2 Groups of 8
A609	6	4	1'0"		Curb Dowels
BENT BARS					
A401	4	14	4'3"		Pads
A402	4	14	5'2"		Pads
A403	4	4	26'5"		Stem
A404	4	4	27'7"		Stem
A407	4	1	3'2"		Wingwall
A408	4	1	5'4"		"
A409	4	5	6'10"		"
A410	4	1	2'8"		"
A411	4	1	4'6"		"
A412	4	5	4'8"		Wingwall
A413	4	4	25'5'2"		Backwall
A414	4	4	27'5'8"		Backwall
A425	4	6	6'2'1"		Guard Rail End Post
A426	4	6	4'3"		Guard Rail End Post
A502	5	34	6'6"		Stem
A608	6	32	3'6"		Approach Slab Seat
ABUTMENT 2					
STRAIGHT BARS					
A423	4	16	24'6"		Backwall
A521	5	68	4'4"		Backwall
A522	5	34	3'0"		Stem & Backwall
A523	5	34	1'13"		Footings & Stem
A524	5	34	2'6"		Dowels
A526	5	14	25'6"		Stem
A527	5	14	27'2"		Stem
A528	5	2	7'4"		Wingwall
A529	5	2	5'6"		Wingwall
A532	5	10	5'0"		Stem & Wingwall
A533	5	10	6'0"		Wingwall
A534	5	32	1'4" to		
			7'4"	4'3"	" 2 Groups of 16
A535	5	4	3'6"		"
A536	5	4	6'6"		"
A537	5	2	9'6"		"
A538	5	2	17'0"		Bend in field
A539	5	7	7'9"		"
A540	5	2	9'6"		Bend in field
A541	5	16	1'7" to		"
			5'6"	6'3"	" 2 Groups of 8
A542	5	6	15'9"		Wingwall

MARK	SIZE	NUMBER	LENGTH	INCR.	LOCATION
ABUTMENT 2 (CONT.)					
STRAIGHT BARS					
A543	5	6	16'9"		Wingwall
A544	5	7	8'9"		Wingwall
A621	6	4	1'0"		Curb Dowels
A623	6	18	6'4"		Footings
A624	6	16	26'0"		"
A625	6	16	15'0" to		"
			26'6"	1'7'3"	" 2 Groups of 8
A626	6	14	13'6"		"
A627	6	14	14'5" to		"
			18'6"	7'2"	" 2 Groups of 7
A628	6	20	5'6" to		"
			9'0"	4'2"	" 2 Groups of 10
A629	6	24	5'6" to		"
			7'10"	2'2"	" 2 Groups of 12
A630	6	6	7'6"		"
A631	6	2	8'0"		"
A632	6	2	8'6"		"
A633	6	2	9'0"		Footings
A634	6	24	8'9"		Wingwall
A635	6	18	10'9"		"
A636	6	10	7'3"		Wingwall
BENT BARS					
A423	4	6	6'2'1"		Guard Rail End Post
A424	4	6	4'3"		Guard Rail End Post
A421	4	14	4'3"		Pads
A422	4	14	5'2"		Pads
A525	5	34	1'1'7"		Stem
A530	5	20	3'0"		Stem & Wingwall
A531	5	10	4'9"		Stem & Wingwall
A622	6	32	3'6"		Approach Slab Seat
APPROACH SLABS					
STRAIGHT BARS					
A5401	4	80	23'8"		Totals are for two approach slabs
Approach Slabs					
A5601	6	344	14'6"		Approach Slabs
PIER 1					
STRAIGHT BARS					
P601	6	53	9'6"		Footings
P602	6	20	26'10"		Footings
P603	6	102	3'10"		Dowels
P604	6	102	19'3"		Plinth
P610	6	5	20'9"		Plinth
P611	6	5	19'6"		Plinth
P616	6	8	26'0"		Cap
P901	9	64	5'8"		Column Dowels
P902	9	64	16'9"		Columns
P903	9	5	39'0"		Cap
P1104	11	8	27'5"		Cap
P1105	11	2	23'0"		"
P1106	11	4	13'9"		Cap
BENT BARS					
P401	4	56	11'4"		Column Ties
P501	5	84	13'4"		Stirrups
P502	5	4	13'2"		"
P503	5	4	12'8"		"
P504	5	4	12'2"		"
P505	5	4	11'8"		"
P506	5	4	11'2"		"
P507	5	4	10'8"		Stirrups
P606	6	40	25'11"		Plinth
P607	6	40	19'10" to		"
			24'8"	3"	" 2 Groups of 20
P612	6	40	4'6"		"
P613	6	10	5'0"		"
P614	6	41	6'1"		Plinth
P615	6	8	10'0"		Cap

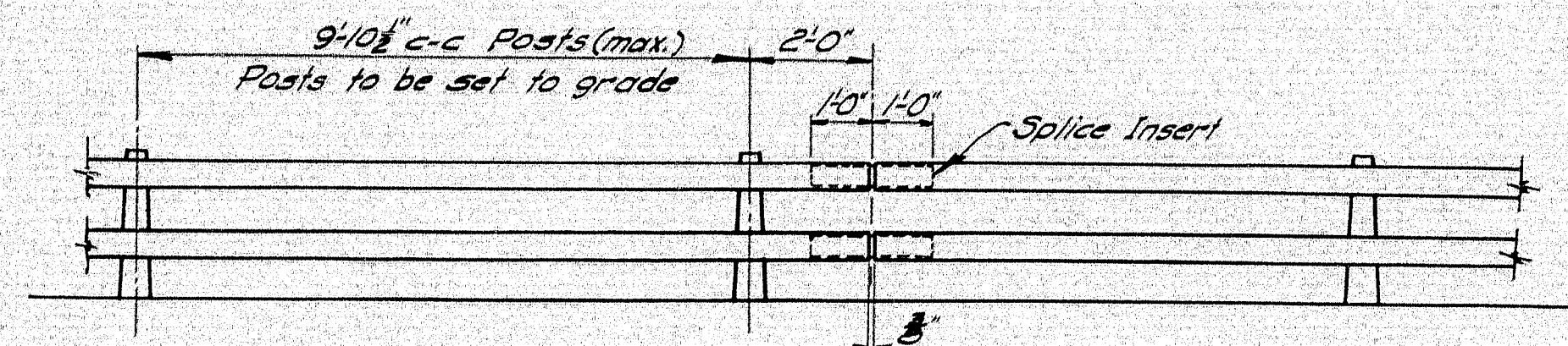
MARK	SIZE	NUMBER	LENGTH	INCR.	LOCATION
PIER 2					
STRAIGHT BARS					
P601	6	53	9'6"		Footings
P602	6	20	26'10"		Footings
P603	6	100	3'10"		Dowels
P605	6	100	15'4"		Plinth
P610	6	5	20'9"		"
P611	6	5	19'6"		Plinth
P616	6	8	26'0"		Cap
P903	9	5	39'0"		Cap
P1101	11	64	7'0"		Column Dowels
P1102	11	48	17'2"		Column
P1103	11	16	9'0"		Column
P1104	11	8	27'5"		Cap
P1105	11	2	23'0"		"
P1106	11	4	13'9"		Cap
BENT BARS					
P401	4	56	11'4"		Column Ties
P501	5	84	13'4"		Stirrups
P502	5	4	13'2"		"
P503	5	4	12'8"		"
P504	5	4	12'2"		"
P505	5	4	11'8"		"
P506	5	4	11'2"		"
P507	5	4	10'8"		Stirrups
P608	6	32	25'3"		Plinth
P609	6	32	20'6" to		"
			24'4"	3"	" 2 Groups of 16
P612	6	32	4'6"		"
P613	6	10	5'0"		"
P614	6	41	6'1"		Plinth
P615	6	8	10'0"		Cap
PIER 3					
STRAIGHT BARS					
P616	6	8	26'0"		Cap
P617	6	60	5'6"		Footings
P618	6	36	7'6"		Footings
P1101	7	16	7'6"		Footings
P901	9	48	5'8"		Dowels
P903	9	5	39'0"		Cap
P904	9	12	27'4"		Column
P905	9	12	28'0"		"
P906	9	12	25'0"		"
P907	9	12	26'6"		Column
P1104	11	8	27'5"		Cap
P1105	11	2	23'0"		"
P1106	11	4	13'9"		Cap
BENT BARS					
P401	4	97	11'4"		Column Ties
P501	5	84	13'4"		Stirrups
P502	5	4	13'2"		"
P503	5	4	12'8"		"
P504	5	4	12'2"		"
P505	5	4	11'8"		"
P506	5	4	11'2"		"
P507	5	4	10'8"		Stirrups
P615	6	8	10'0"		Cap
SUPERSTRUCTURE					
STRAIGHT BARS					
S402	4	40	1'5"		End Post
S504	5	354	21'10"		Top Transverse


MARK	SIZE	NUMBER	LENGTH	INCR.	LOCATION
SUPERSTRUCTURE (CONT.)					
STRAIGHT BARS					
S505	5	354	29'7"		Top Transverse
S506	5	354	17'10"		Bottom Transverse
S507	5	354	35'7"		Bottom Transverse
S508	5	150	32'0"		Longitudinal B
S509	5	424	30'8"		Longitudinal A-1
S510	5	553	38'8"		Longitudinal B & A-2
S511	5	32	14'11"		Sidewalk A-1
S512	5	40	19'8"		Sidewalk B & A-2
S513	5	20	31'4"		Longitudinal A-1
BENT BARS					
S401	4	16	8'7"		End Post
S501	5	352	17'0"		Truss Rod
S502	5	352	36'23"		Truss Rod
S503	5	708	5'0"		Sidewalk



NOTES:
1. All dimensions are to E of bars.
2. All reinforcing bars shall be intermediate grade steel.
3. Reinforcing steel to have 2" minimum cover, unless otherwise shown.
4. Minimum length of splice is 17 bar diameters.

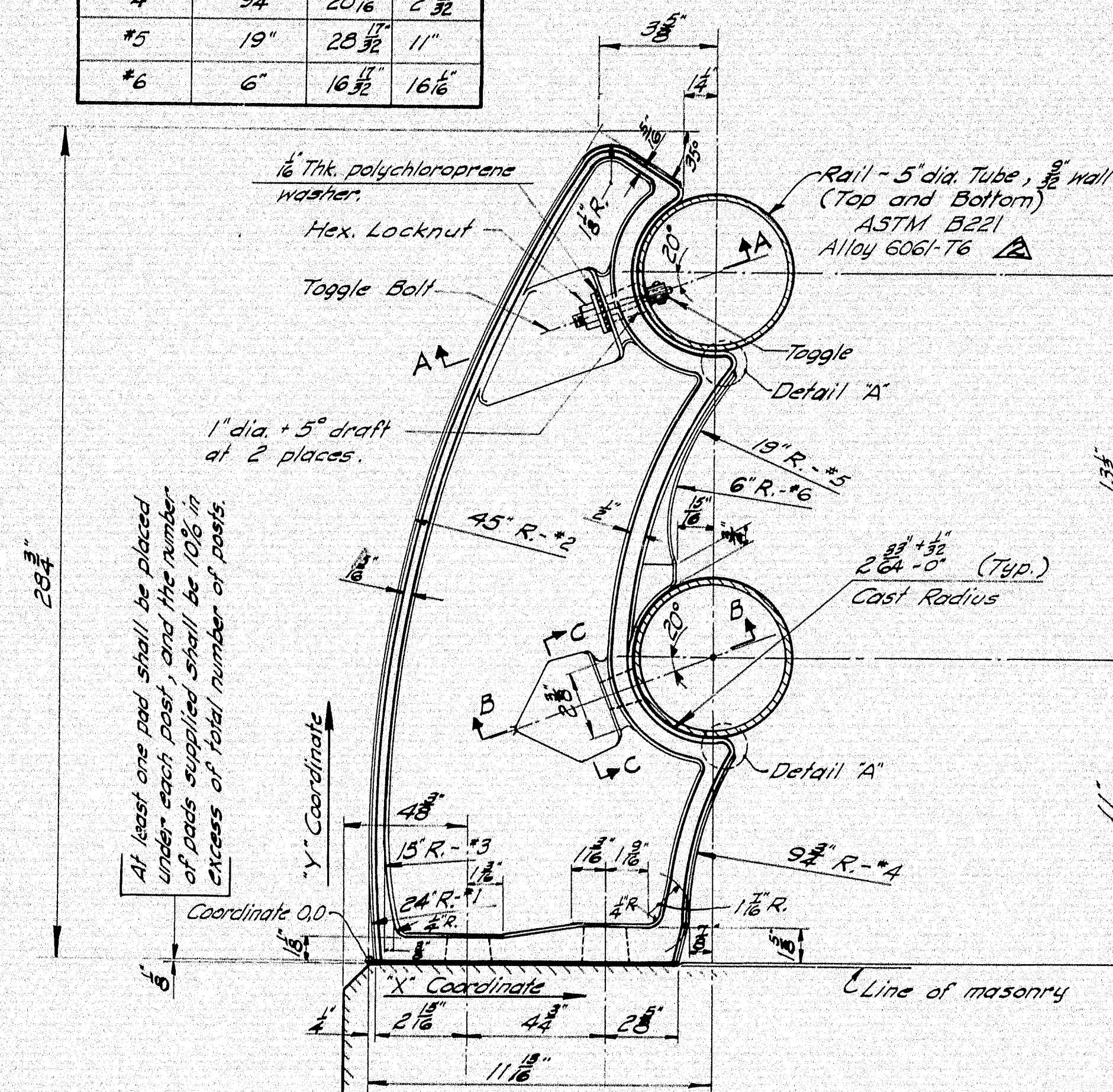
DESIGN - HOWARD, NEEDLES, TAMMEN & BERGENDOFF
TRACE - CHECK - PRN.
DETAIL - RWOL.
BRIDGE NO. SURVEY - PLOT -
STATE HIGHWAY COMMISSION
BRIDGE DIVISION
INTERSTATE 95 S.B.
OVER
FISH STREAM & RELOC. ROUTE S.A. NO. 1
IN THE TOWN OF
ISLAND FALLS
ARROSTOCK COUNTY
REINFORCING STEEL
SHEET 12 OF 12 AUGUSTA, MAINE AUGUST 1965

RAIL ELEVATION

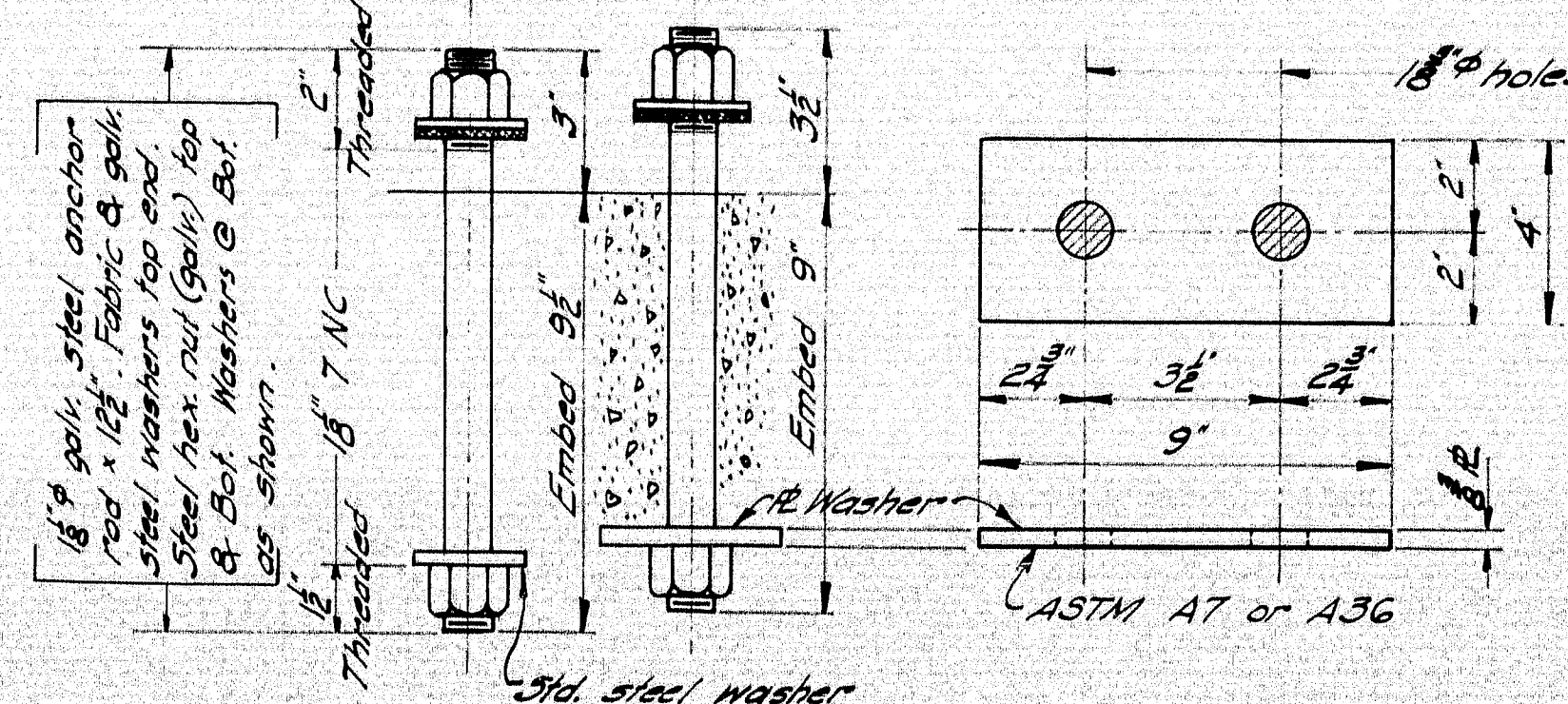
 ORIGIN LOCATION-PRINCIPAL CURVES

Curve	Radius	"X"	"Y"
*1	24"	24"	3 $\frac{15}{32}$
*2	45"	45"	2 $\frac{27}{32}$
*3	15"	15 $\frac{11}{16}$	4 $\frac{29}{32}$
*4	9 $\frac{3}{4}$	20 $\frac{11}{16}$	2 $\frac{13}{32}$
*5	19"	28 $\frac{17}{32}$	11"
*6	6"	16 $\frac{17}{32}$	16 $\frac{1}{32}$

Lengths of rail shall be attached to a minimum of (4) four rail posts, wherever possible, and in any case never less than (2) two.

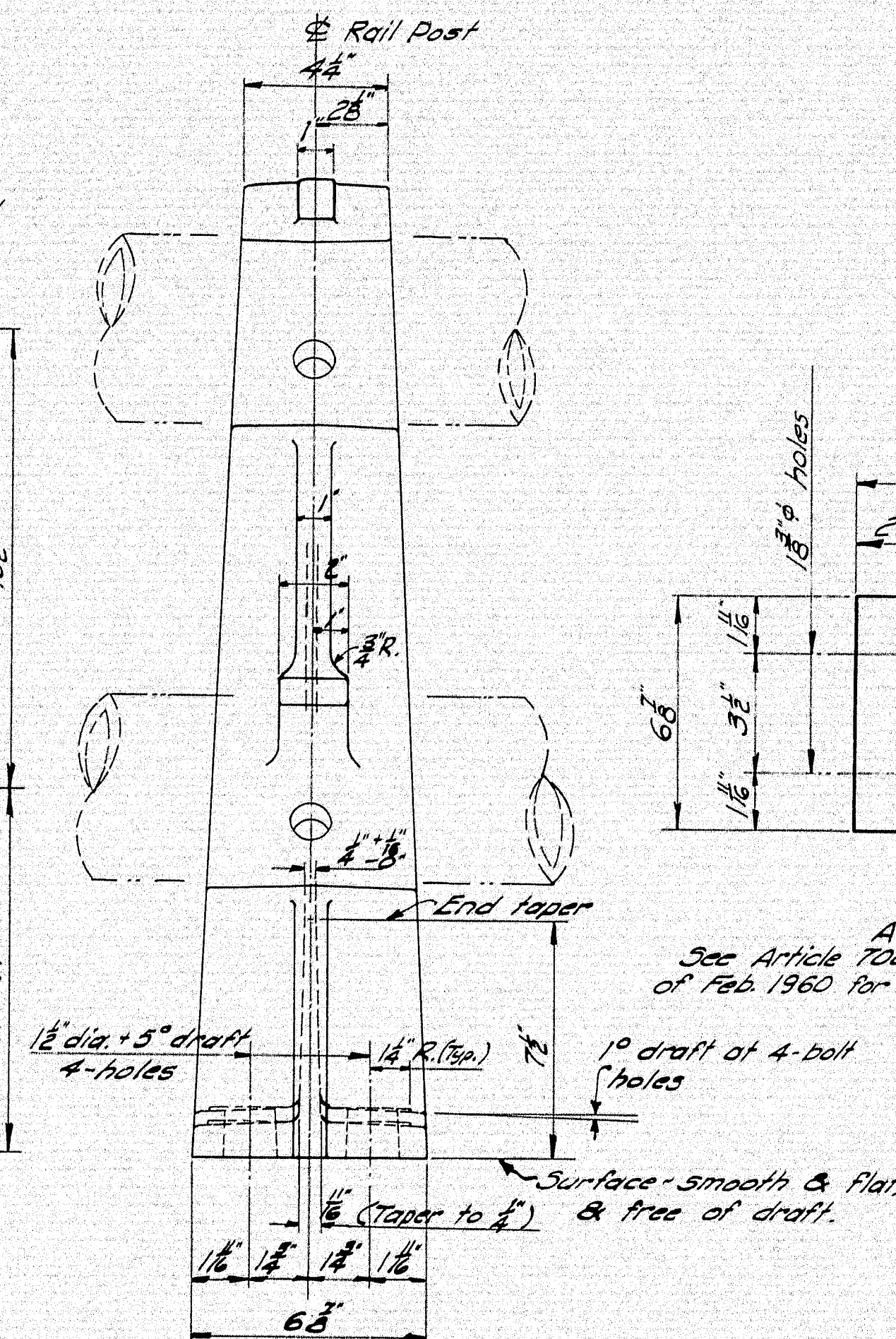
RAIL POST

Aluminum Association Alloy A344-T4

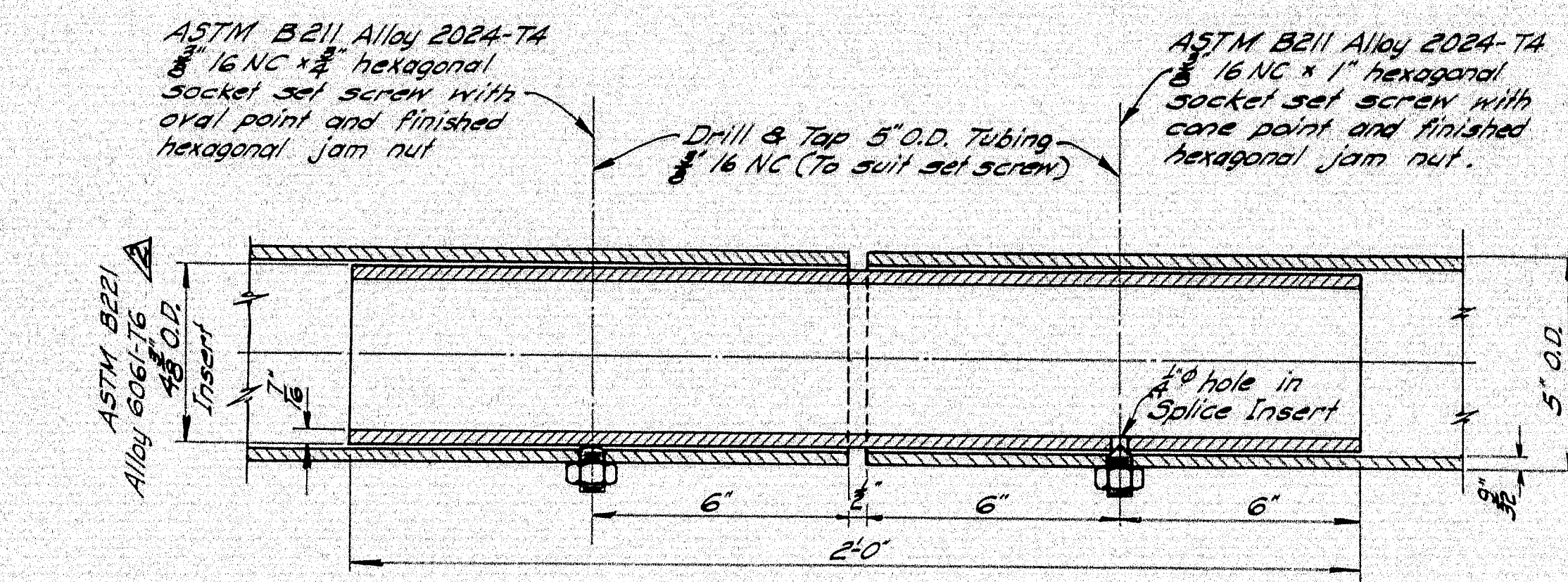
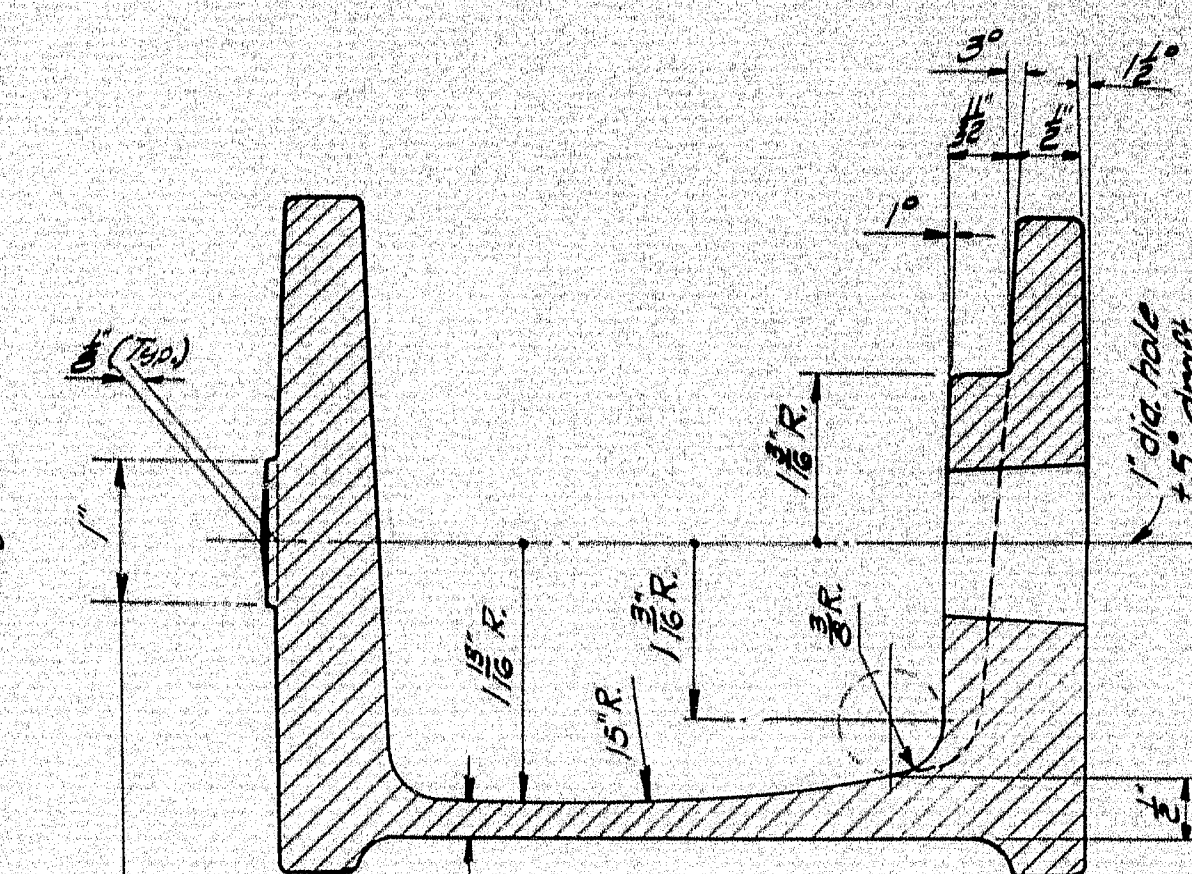


RAIL POST ANCHORAGE

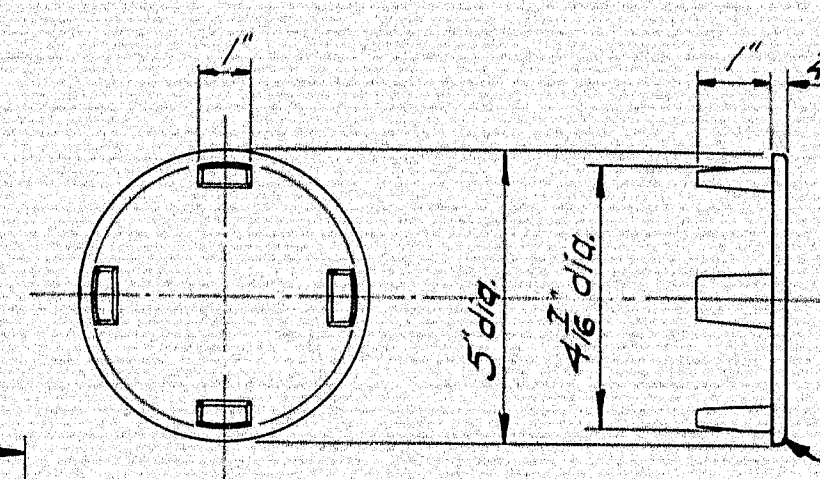
Bolts, Nuts & Std. Washers = ASTM A325 Galvanized ASTM A153



FRONT ELEVATION

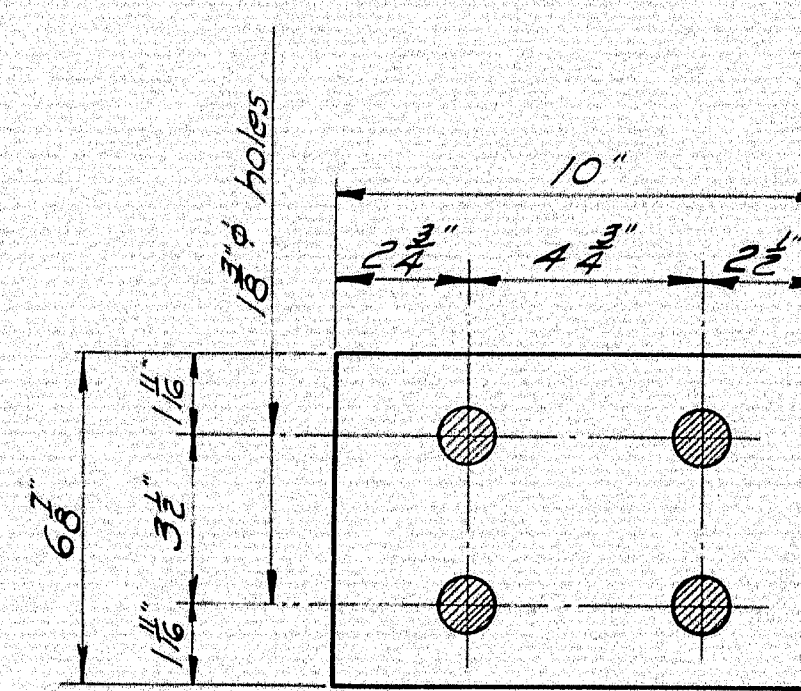
SPLICE

SECTION A-A



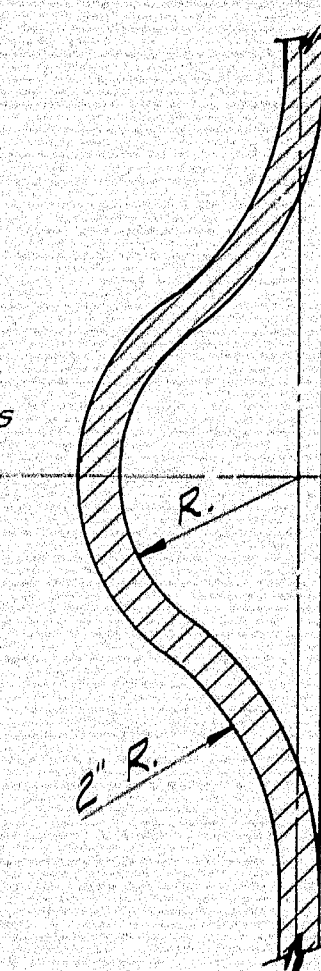
RAIL CAP

ASTM B26 Alloy SG 70 A or S5A

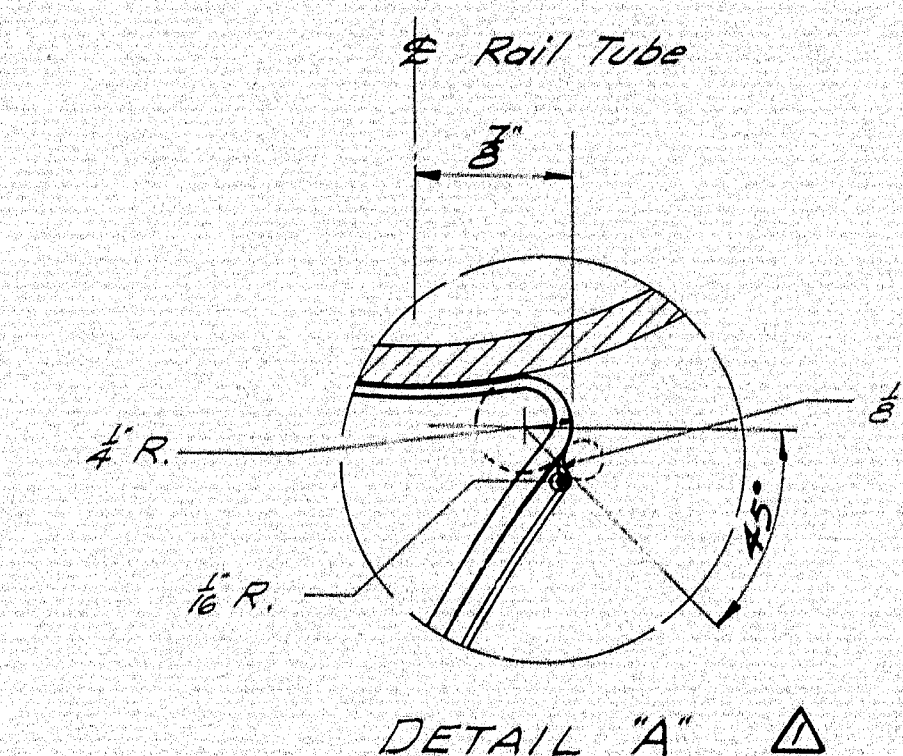


PAL

At each rail post
See Article 702-80, Supplemental Specifications
of Feb. 1960 for Pad & Fabric Washers.



SECTION C-C



DETAIL "A"

SECTION B-B

DESIGN SPECIFICATIONS

A. A. S. H. O. Interim Specifications
Int. I (64)

A 344-T4 Alloy to meet the
Specification outlined by Aluminum
Association.

ALTERATION:

△ - Added Detail "A" and Origin Location-
Principal Curves. Nov. 19, 1964.

△ - Removed Alloy 6062-T6 May 5, 1966

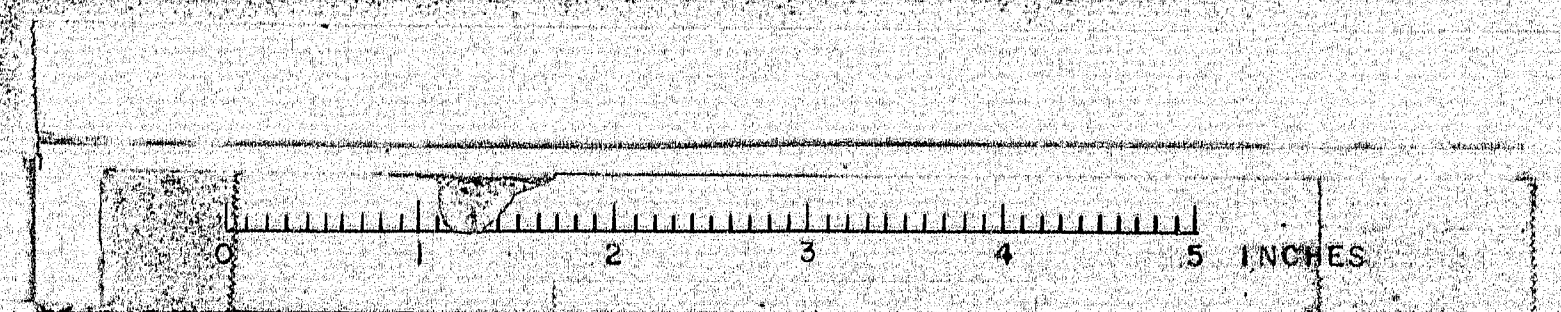
MAINE STATE HIGHWAY COMMISSION
AUGUSTA, MAINE

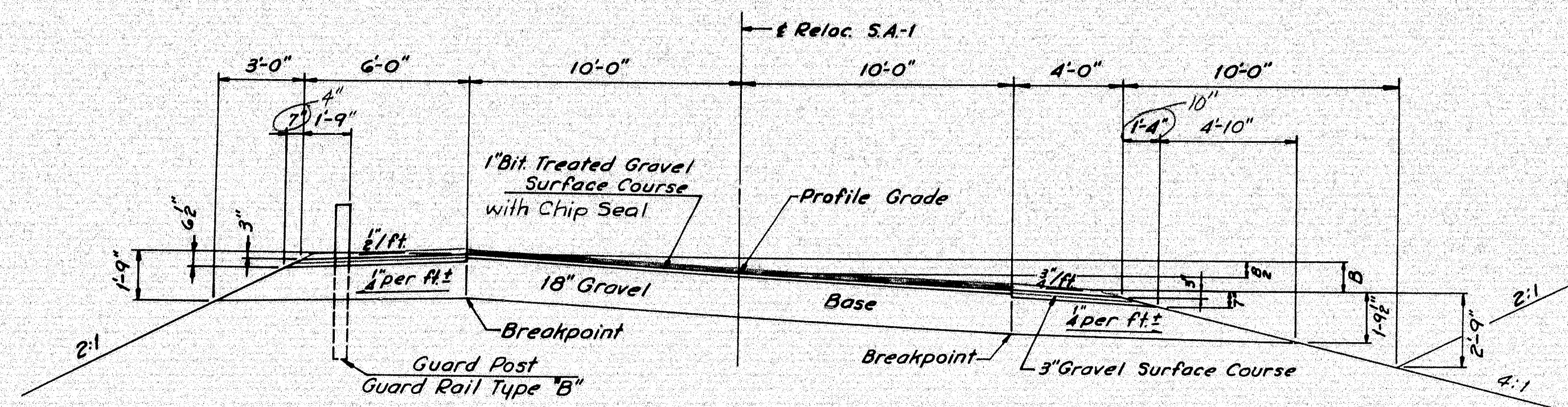
STANDARD DETAILS
(BD 108-64)
ALUMINUM RAIL
2 - BAR (TUBE RAIL)
CAST POST

OCT. 1964

101-216 ISLAND FALLS (43)

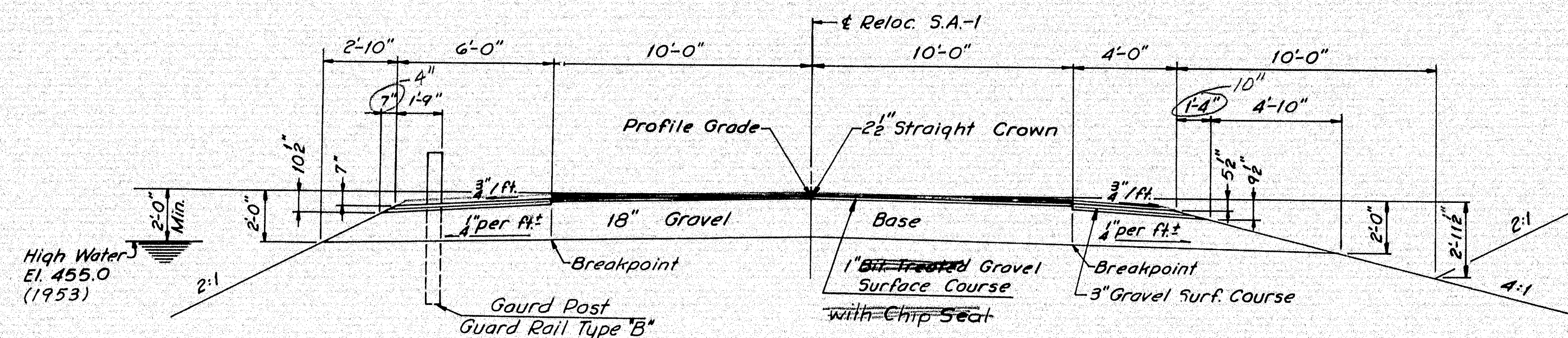
5





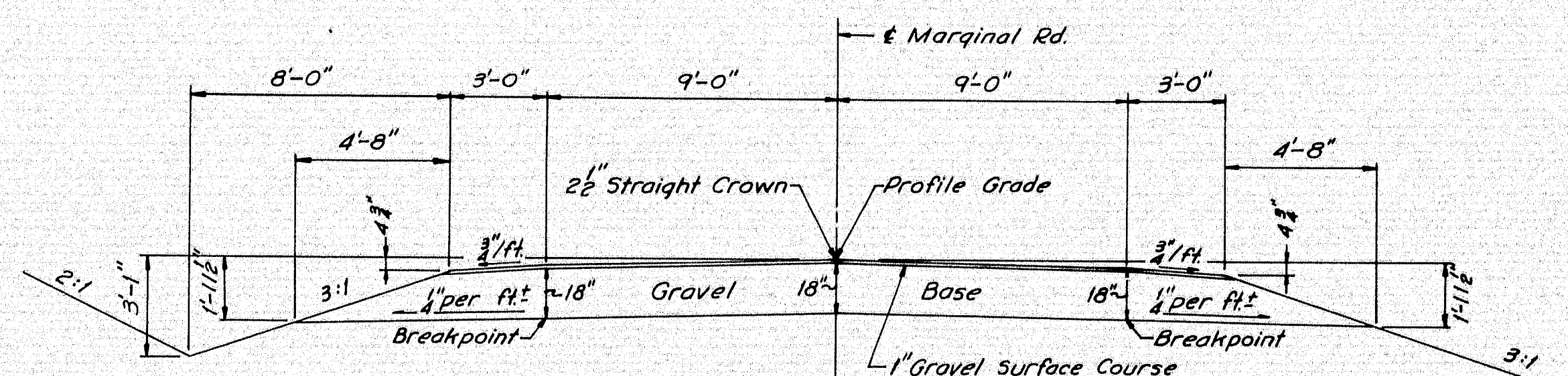
RELOC. S.A.-I
SUPERELEVATED

6 FT. SHOULDER	20 FT. ROADWAY	4 FT. SHOULDER
3" GRAVEL SURFACE = 5.83 C.Y. PER 100 L.F.	1" GRAVEL SURFACE = 6.17 C.Y. PER 100 L.F.	3" GRAVEL SURFACE = 4.32 C.Y. PER 100 L.F.
16" GRAVEL BASE = 35.64 C.Y. PER 100 L.F.	18" GRAVEL BASE = 111.11 C.Y. PER 100 L.F.	16" GRAVEL BASE = 34.05 C.Y. PER 100 L.F.
	9'-00" B = 16"	
	4'-00" B = 10"	



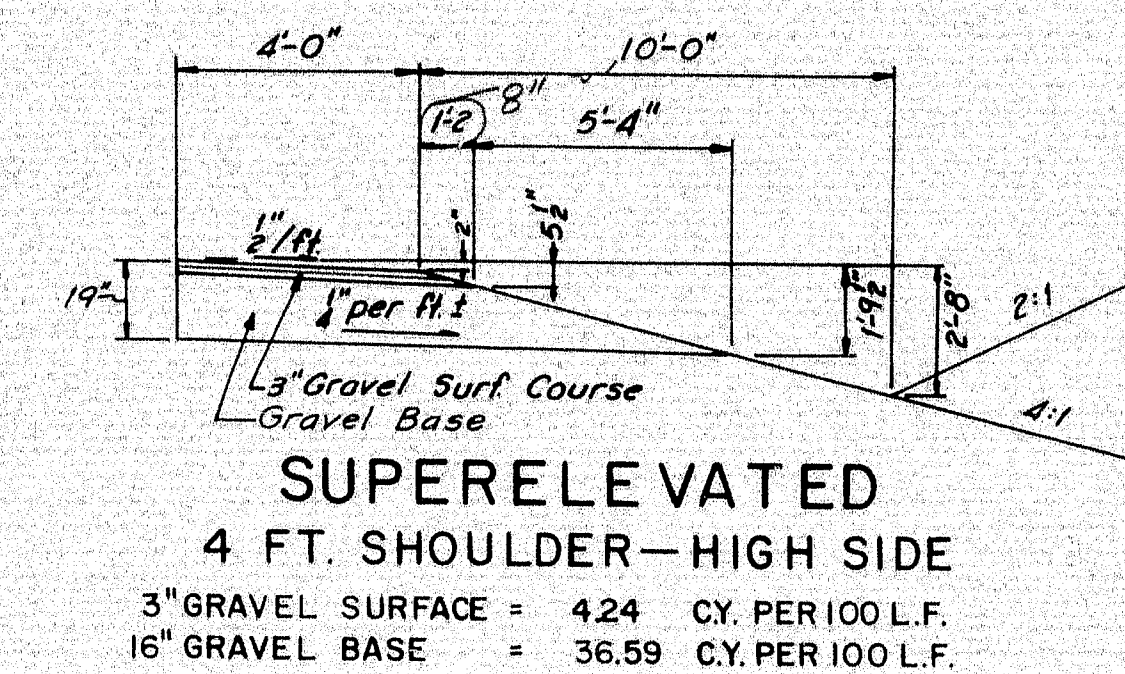
RELOC. S.A.-I
NORMAL

6 FT. SHOULDER	20 FT. ROADWAY	4 FT. SHOULDER
3" GRAVEL SURFACE = 5.83 C.Y. PER 100 L.F.	1" GRAVEL SURFACE = 6.17 C.Y. PER 100 L.F.	3" GRAVEL SURFACE = 4.32 C.Y. PER 100 L.F.
16" GRAVEL BASE = 33.80 C.Y. PER 100 L.F.	18" GRAVEL BASE = 111.11 C.Y. PER 100 L.F.	16" GRAVEL BASE = 34.05 C.Y. PER 100 L.F.



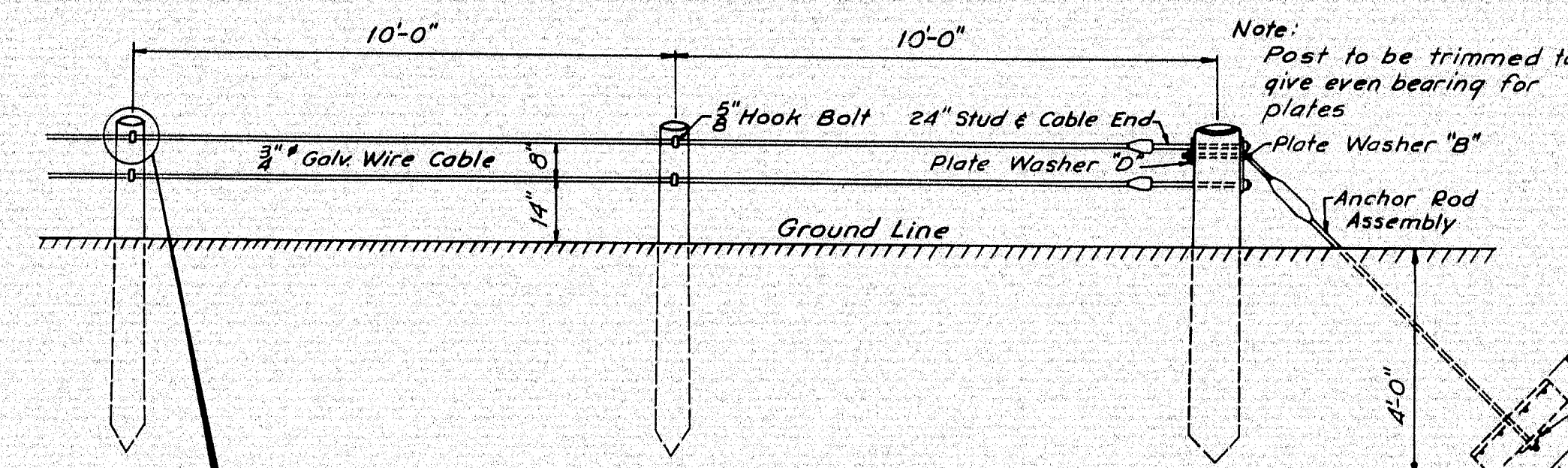
RELOC. MARGINAL ROAD
NORMAL

3 FT. SHOULDER	18 FT. ROADWAY	3 FT. SHOULDER
1" GRAVEL SURFACE = 0.96 C.Y. PER 100 L.F.	1" GRAVEL SURFACE = 5.56 C.Y. PER 100 L.F.	1" GRAVEL SURFACE = 0.96 C.Y. PER 100 L.F.
16" GRAVEL BASE = 28.38 C.Y. PER 100 L.F.	18" GRAVEL BASE = 100.00 C.Y. PER 100 L.F.	16" GRAVEL BASE = 28.38 C.Y. PER 100 L.F.



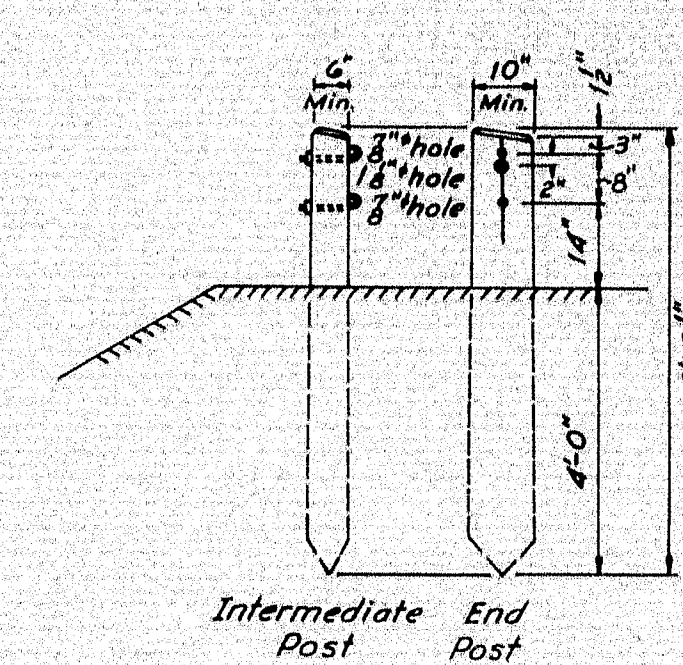
SUPERELEVATED
4 FT. SHOULDER-HIGH SIDE

3" GRAVEL SURFACE = 4.24 C.Y. PER 100 L.F.
16" GRAVEL BASE = 36.59 C.Y. PER 100 L.F.



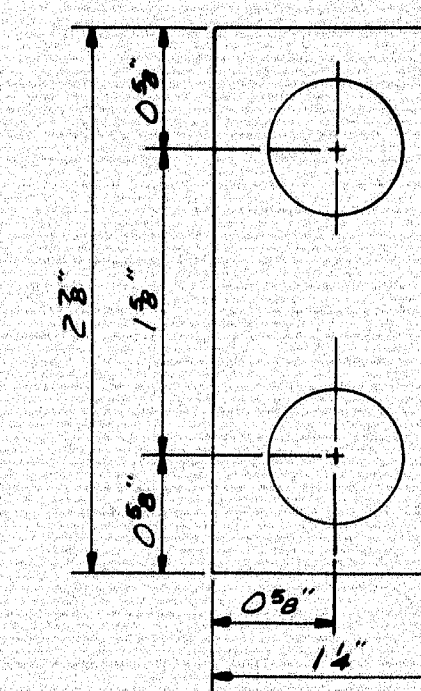
HOOK BOLT PLATE DETAIL
GUARD RAIL TYPE "B"

NOTES:
Superelevation shall be as specified in AASHO Policy of Geometric Design of Rural Highways (e). The maximum superelevation shall not exceed 0.08' per foot width of pavement. All curves shall have full superelevation at the first 50' station occurring a minimum distance of 50' after the P.C. and a minimum distance of 50' before the P.T. When superelevation exceeds 1% per foot, inside shoulder shall have same slope as pavement.
For all sections depth of ditch depends on local conditions. Depth of base as shown may change to meet local conditions.
The pavement and base depths as shown on the plans are intended to be nominal.



Type "B" Anchor Posts shall be Norway or Southern Yellow Pine, all others shall be Norway Pine, Southern Yellow Pine, Pitch Pine, or Douglas Fir (pressure treated).

Reinforced Concrete Anchor 24"x24"x7" with four #4 bars each way or a Granite Block, min. face area 4 sq. ft. & min. thickness 6".



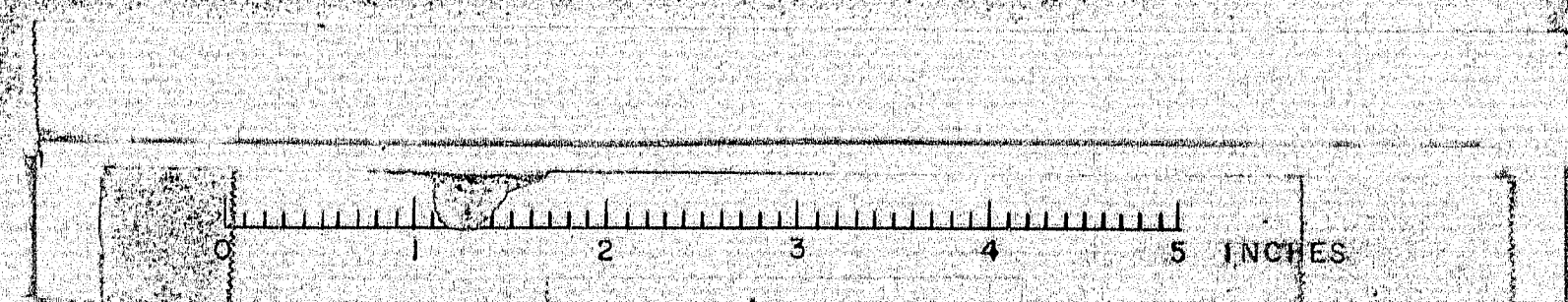
Note: Thickness 12 Ga. = 0.1046". Holes may be drilled or punched 3/8" before galvanizing.

MAINE STATE HIGHWAY COMMISSION
AUGUSTA, MAINE

TYPICAL SECTIONS

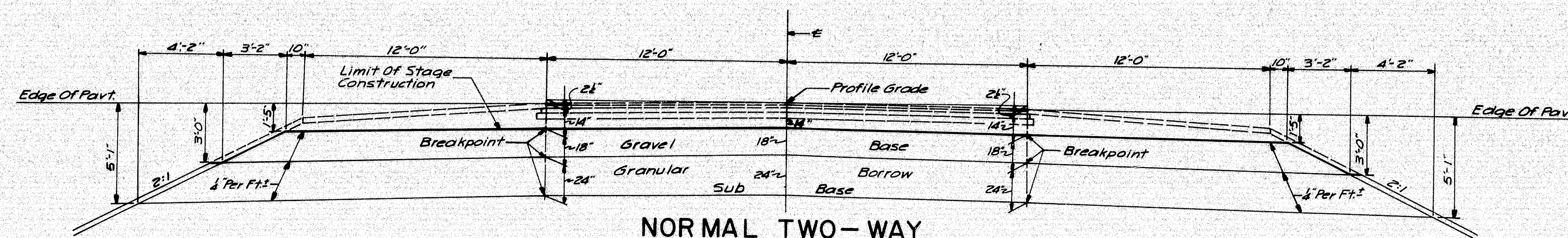
HOWARD, NEEDLES, TAMMEN & BERGENDOFF
CONSULTING ENGINEERS
NEW YORK BOSTON KANSAS CITY

101-217 ISLAND FALLS (43)



STAGE CONSTRUCTION - GRADING AND GRAVEL BASE*

18" GRAVEL BASE
24" GRANULAR BORROW SUBBASE



12 FT. SHOULDER

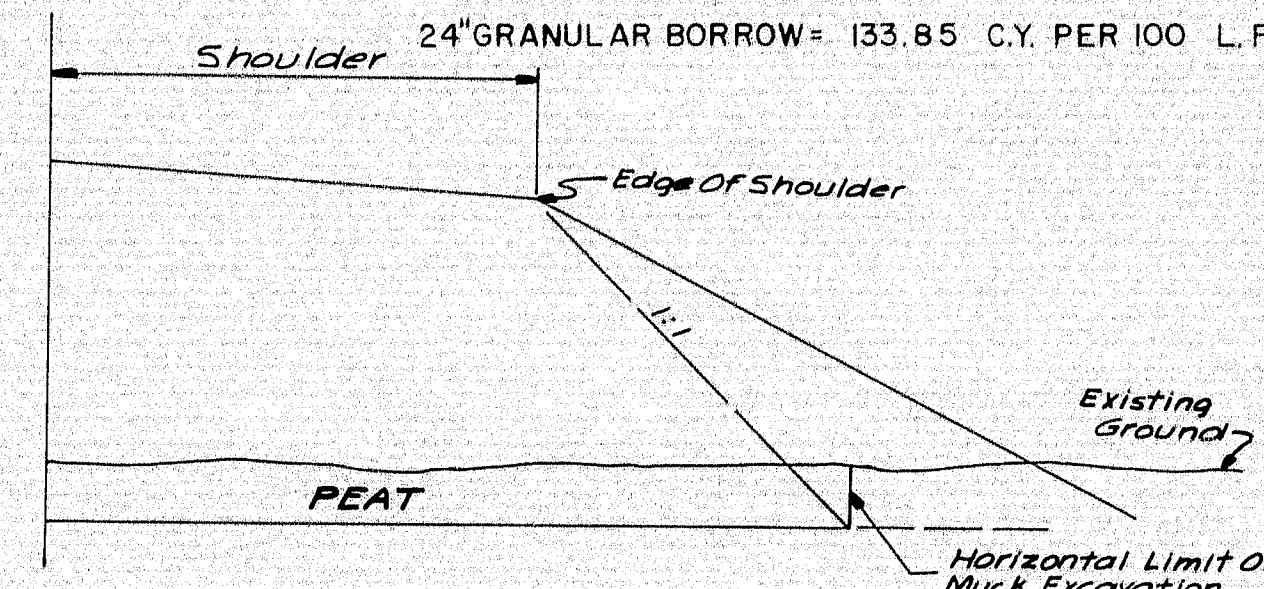
24 FT. PAVEMENT

12 FT. SHOULDER

18" GRAVEL BASE = 80.61 C.Y. PER 100 L.F.
24" GRANULAR BORROW = 133.85 C.Y. PER 100 L.F.

18" GRAVEL BASE = 133.33 C.Y. PER 100 L.F.
24" GRANULAR BORROW = 177.78 C.Y. PER 100 L.F.

18" GRAVEL BASE = 80.61 C.Y. PER 100 L.F.
24" GRANULAR BORROW = 133.85 C.Y. PER 100 L.F.

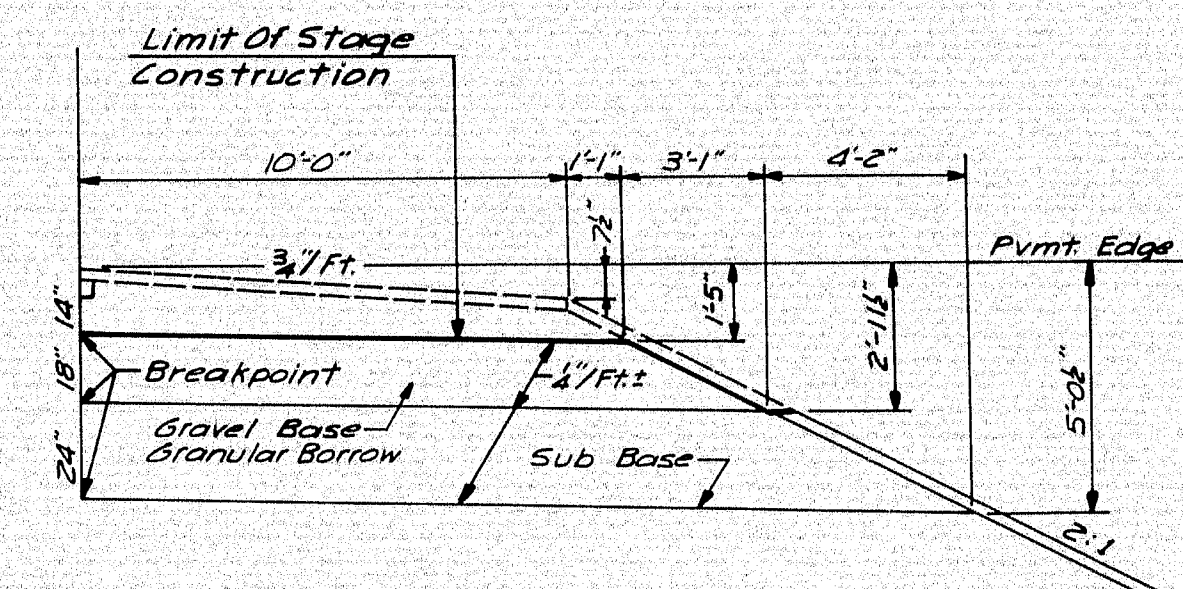


DETAIL OF MUCK EXCAVATION

NOTES:

* The Pavement And Base Depths As Shown On The Plans Are Intended To Be Nominal

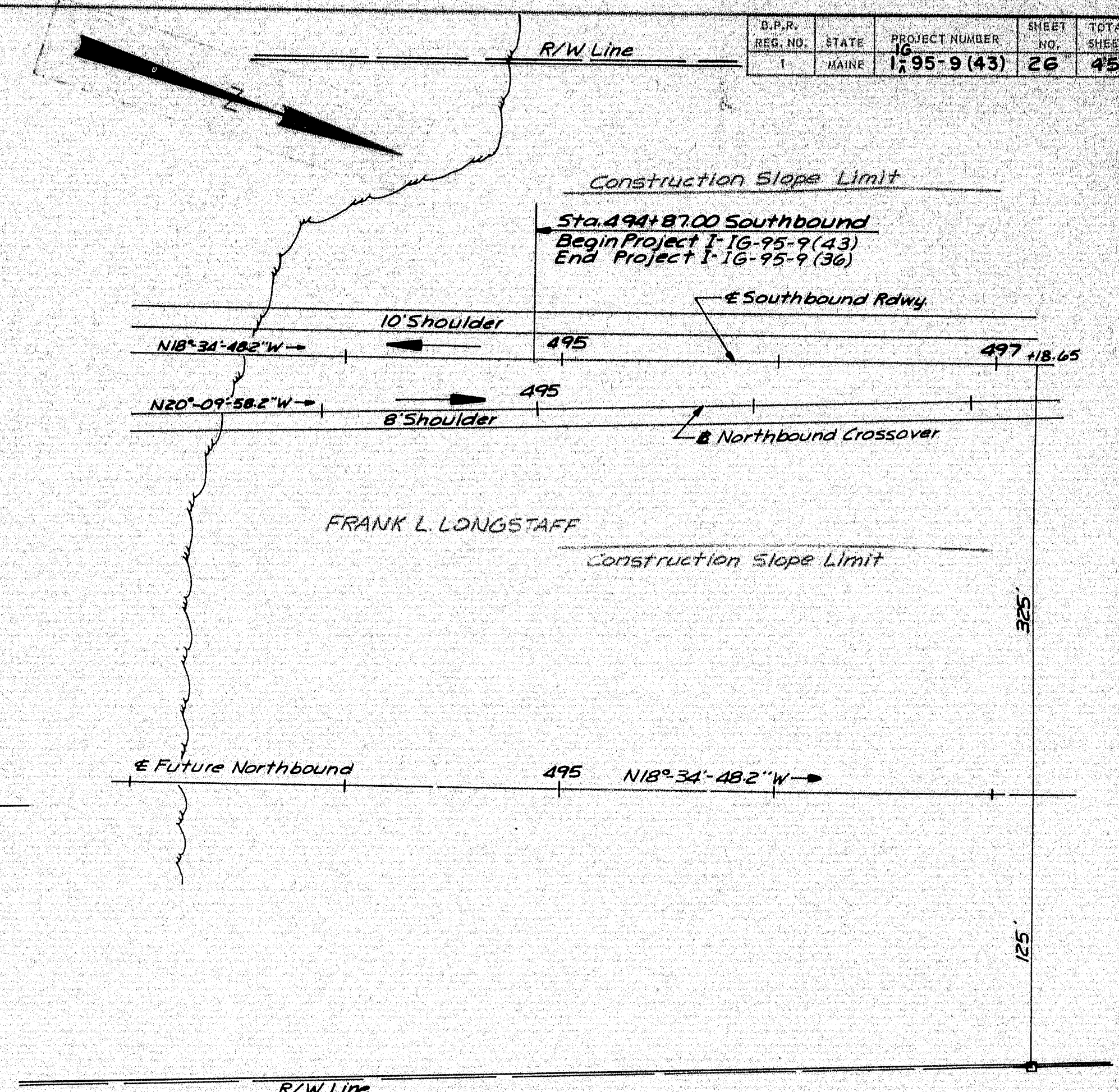
When Slopes Are Specified To Be Loamed Either By The Engineer Or Notes On The Plans Or Sections, They Shall Be Loamed Up To The Bottom Of Gravel Base



CROSSOVER

10 FT. SHOULDER - LOW SIDE

18" GRAVEL BASE = 69.57 C.Y. PER 100 L.F.
24" GRANULAR BORROW = 120.31 C.Y. PER 100 L.F.



GENERAL NOTES

1. ALL LOAM AREAS AND DEPTHS MUST BE AUTHORIZED BY THE ENGINEER UNLESS SPECIFICALLY CALLED FOR ON THE TYPICAL SECTIONS, PLANS OR IN THE SPECS. LOAMING OF SLOPES HAS BEEN ESTIMATED ON A 2' DEPTH
2. ALL SLOPES SHALL BE LOAMED EXCEPT SA-1 AND MARGINAL RD UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
3. ALL 5:0:1 ROADWAY EARTH BACKSLOPES SHALL BE 3:1 UNLESS OTHERWISE SHOWN.
4. ALL UTILITY FACILITIES ARE TO BE ADJUSTED AS NECESSARY BY THE RESPECTIVE UTILITIES UNLESS NOTED.
5. THE UTILITIES INVOLVED IN THIS CONTRACT ARE THE MAINE PUBLIC SERVICE CO., THE KATAHDIN FARMERS TELEPHONE CO. AND THE B. & A. R. R.
6. SEEDING METHOD NO. 2 AND HAY MULCH ON ALL SLOPES AS DIRECTED BY THE ENGINEER.
7. ALL MUCK EXCAVATION AREAS SHALL BE BACKFILLED TO AN ELEVATION OF 1' ABOVE EXISTING GROUND WITH GRANULAR BORROW OR EXCAVATION MEETING THE REQUIREMENTS FOR GRANULAR BORROW EMBANKMENT CONSTRUCTION.
8. UNLESS OTHERWISE DIRECTED BY THE ENGINEER THE GRUBBING IN FILL WIDTHS ARE AS FOLLOWS:
SA-1 = 20' EACH SIDE OF E.
MARGINAL RD. = 15' EACH OF E.
9. INSTALL JUTE MATTING IN ALL INLET AND OUTLET DITCHES AS DIRECTED BY THE ENGINEER.
10. ALL EXCAVATION DEEMED NECESSARY BY THE ENGINEER FOR THE INSTALLATION OF HAND LAID RIP-RAP AND/OR DOWNSPOUTS WHERE EXCAVATION IS NOT PAID FOR UNDER ANOTHER ITEM SHALL BE PAID FOR UNDER ITEM 202.10 STRUCT. EARTH EXC. - DRAINAGE OR ITEM 204.11 STRUCT. ROCK EXC. - DRAINAGE.
11. REMOVING TREES, RIGHT OF WAY MONUMENTS AND SURVEY MARKERS TO BE DONE BY OTHERS.

GUARD RAIL SECTIONS

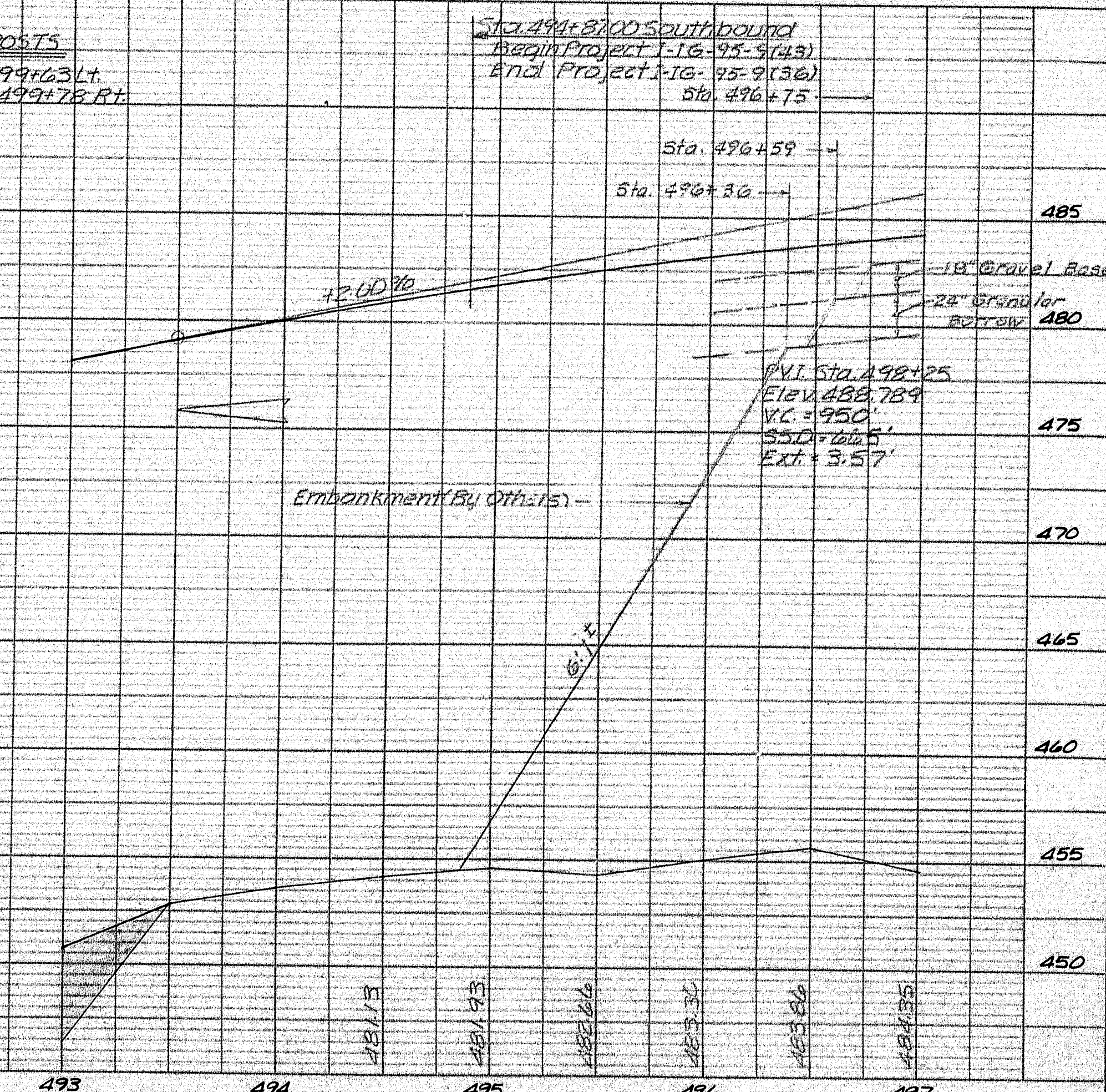
Sta. 494+87 To 499+20 L.H.
Sta. 494+87 To 499+20 R.H.

FENCING - METAL POSTS

Sta. 494+00 To Sta. 499+23 L.H.
Sta. 492+50 To Sta. 499+78 R.H.

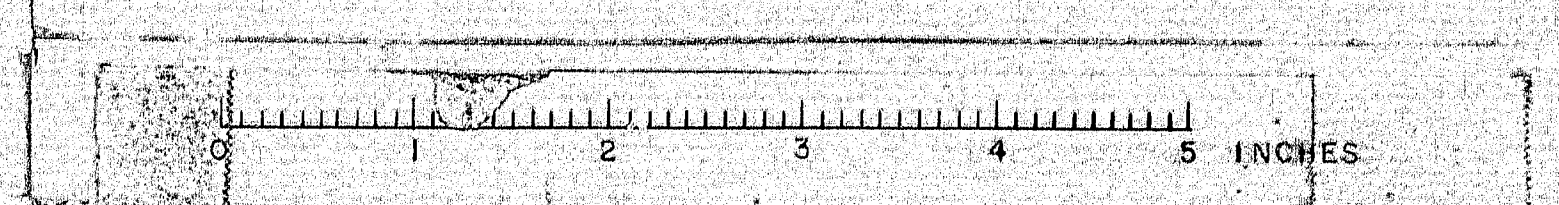
Sta. 494+87 To 499+20 L.H.
Sta. 494+87 To 499+20 R.H.

Sta. 494+00 To Sta. 499+23 L.H.
Sta. 492+50 To Sta. 499+78 R.H.



101-218

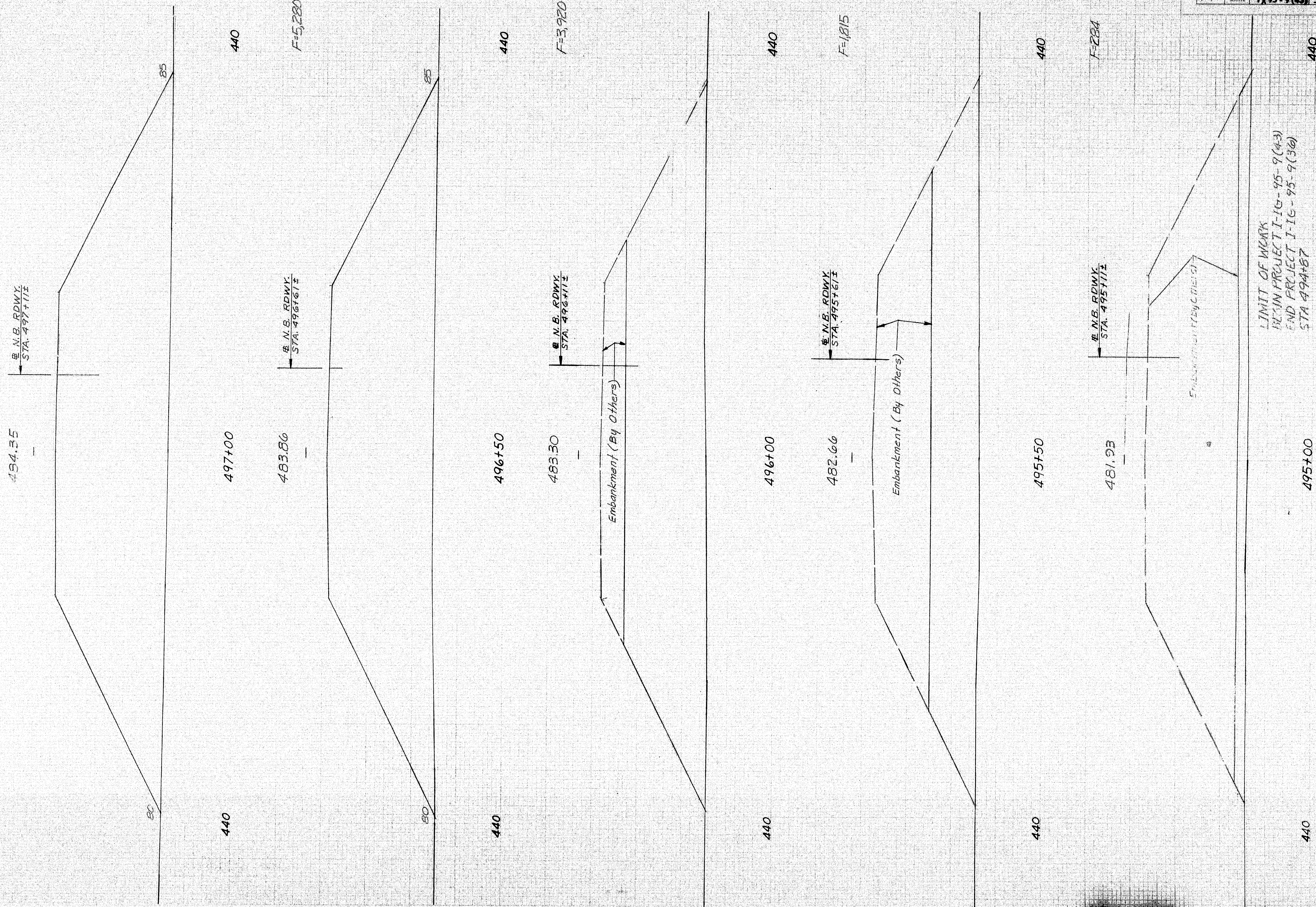
ISLAND FALLS (43)



5.46.1000. 7.94
 2.6.1000. 10.04
 3.4.1000. 12.05

943

4



D.P.R.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	105-9(43)	30	45

S. E. X. SECTIONS 495+00 TO 497+00

BLAND FALLS (43) 89

943

STA. 499+75 ±

STRUCTURE

485.50

499+50
LAST FULL SECTION
STA. 499+20

AND

$E=5.540$

48550

9/2

18

499+00

440

F=5870

48533

19

12

 $498+50$

140

ENBRD
STA 498+11±

48508

$F = 5,900$

5

N

198+00

440

4 N.B. RDWY.
STA 497 + 61

18A-715

11-11-11

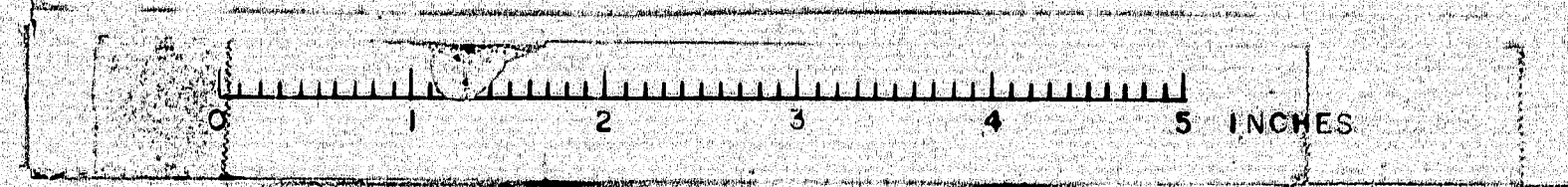
7

七

197+50

440

0774

 $497+50 \quad 499+50$ 

B.P.R. REG. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	11695-9(43)	31	45

3.5% ROAD
3.5%
3.5%

943

485.38

STRUCTURE
FIRST FULL SECTION
STA 501+25

440

501+00

STRUCTURE

MUCK EXCAVATION STA.
500+80± TO STA. 504+00±

440

F=3820
C=68
M=6

440

500+83

STRUCTURE

STA. 500+70
O-SECTION

440

450

500+60

STRUCTURE

450

440

500+27

STRUCTURE

485.60

440

450

500+00

450

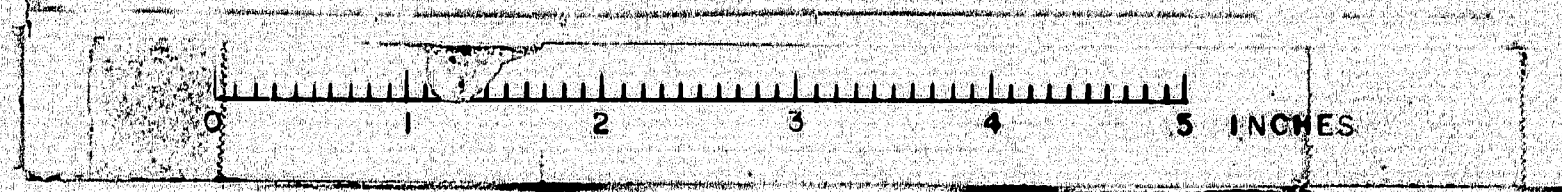


PLATE 5, CROSS SECTION
RUMBLE & BERRY CO.

A.P.A. REG. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	183	32	41

(45)

5.5% ROAD 500+00 501+00

M.A.S.
J.A.H.
D.O.D
M.A.S.
J.A.H.
D.O.D

943

STA. 505+55
C-SECTION

$F=5,810$
 $C=39$

LAST FULL SECTION
STA. 504+90

TO FISH STREAM.

100

449.0

440

100

504+70

22

48299

$F = 8,100$
 $C = 35$

22

4495

440

48

504+00

440

484.00

$$\begin{aligned} F &= 12,600 \\ C &= 716 \\ M &= 570 \end{aligned}$$

90

140

503+00

440

48485

$$\begin{aligned} F &= 13,500 \\ C &= 940 \\ M &= 2,020 \end{aligned}$$

Yes

130

502+00

$$\begin{aligned} F &= 16,400 \\ C &= 79\% \\ M &= 12\% \end{aligned}$$

502+00 504+70

(43

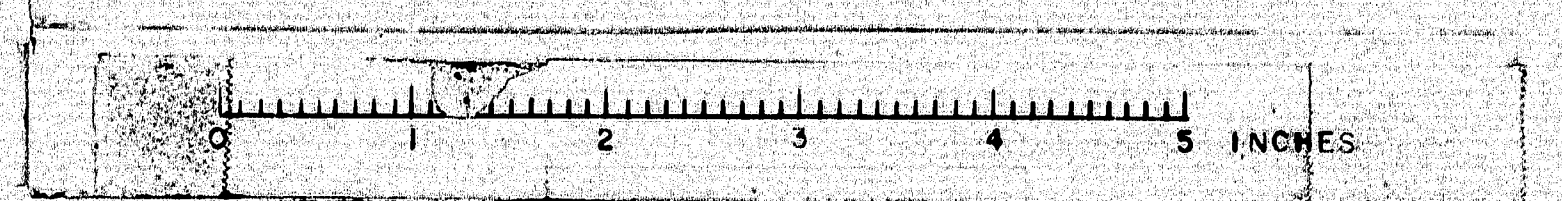


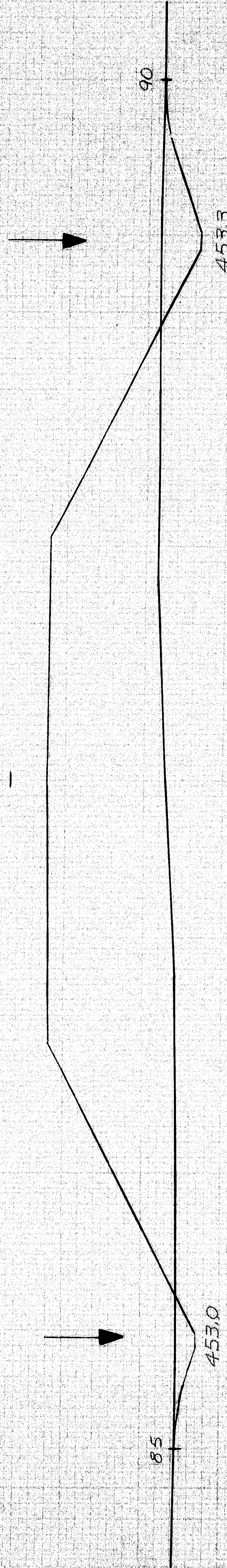
PLATE 3. CROSS SECTION
RIBBED & FLAT CO.

H.P.R. REG. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEET
1	MAINE	159-902	33	43

5445-000 7644
5445-000 7644
5445-000 7644

943

476.95



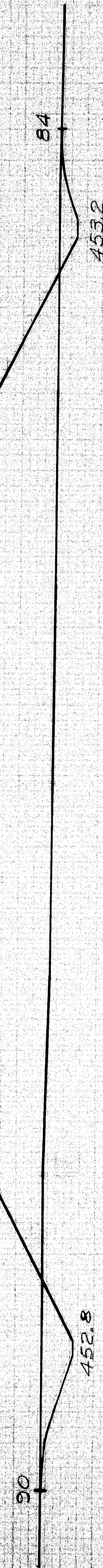
440

510+00

440

477.45

C=153
F=2,830



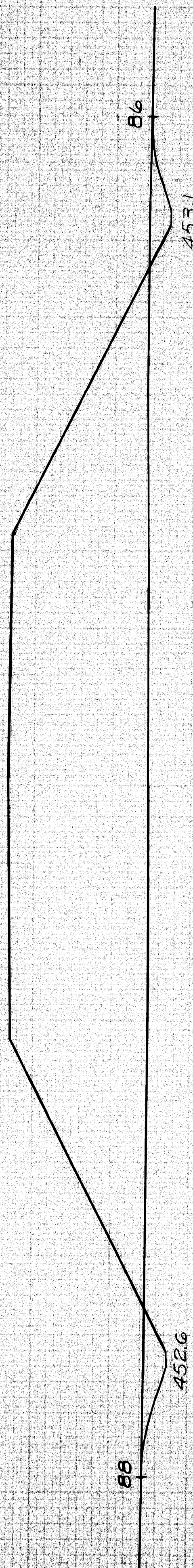
440

509+50

440

477.95

C=106
F=3,440



440

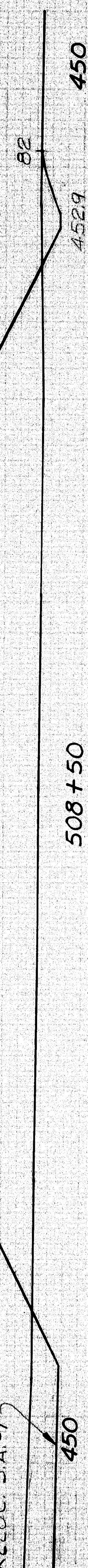
509+00

440

478.46

C=97
F=3,300

DITCH FROM 452.4
RELOC. SA-1



450

508+50

450

FIRST FULL SECTION
STA 508+25
478.96

STRUCTURE

C=61
F=2,845

450

508+00

450

479.47

STRUCTURE
O-SECTION
STA 507+90

450

507+50

450

479.97

STRUCTURE

450

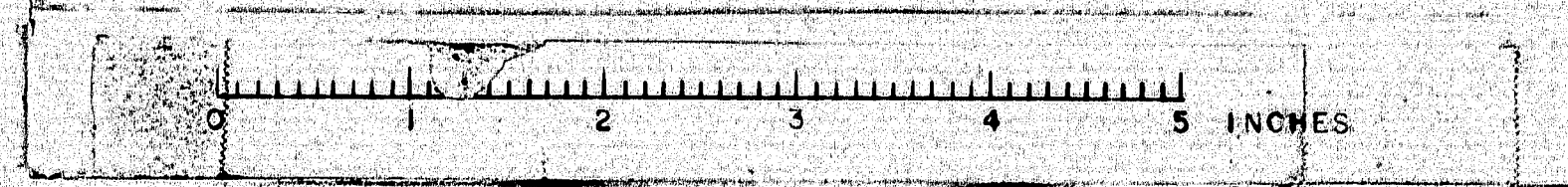
507+00

450

FISH STREAM

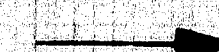
507+00 510+00

B.P.R.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	100-000	54	45

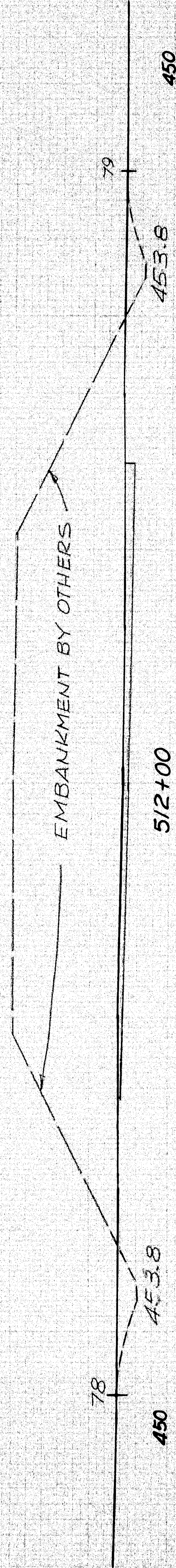


346 F.D.D.D.
20.04
2.03

943

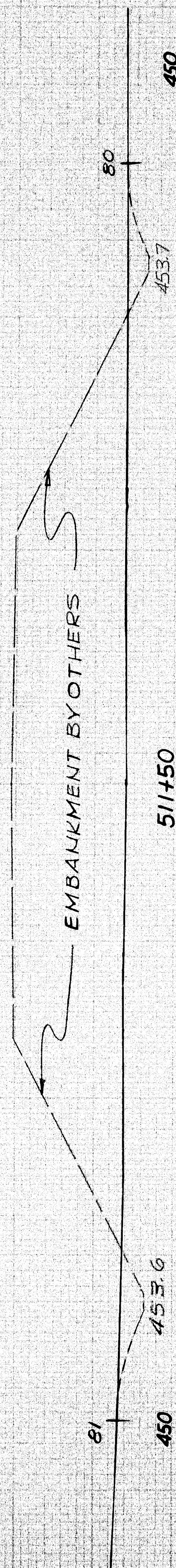


474.93



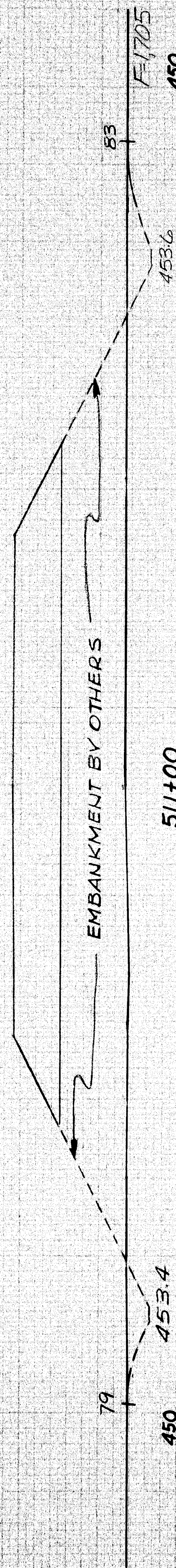
STA. 511+66.00
END PROJECT I-16-95-9(43)
BEGIN PROJECT I-95-9(37)
LIMIT OF WORK

475.43



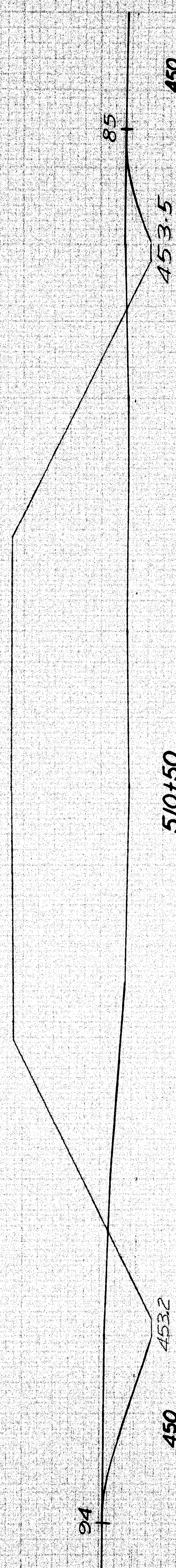
F=320

475.94



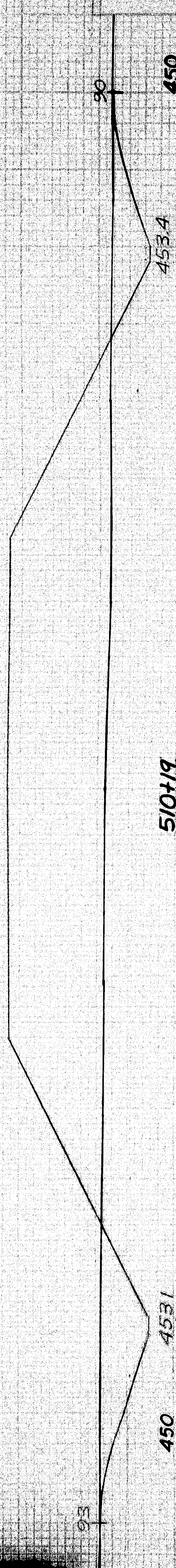
F=1705

476.44

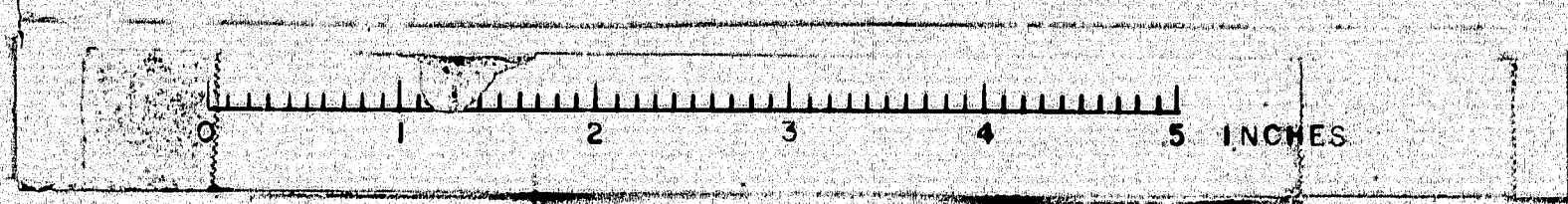


C=190
F=1430

476.94



C=102
F=900



B.P.R.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	1-16-95-9(43)	35	43

510+19 TO 512+00

(43)

S.H.C. & C.E.A.
J.A.M.
C.E.A.

943

±

458.44

-8" @ 2+60

+8" @ 2+60

A.1

49

31

445

2+50

445

C=14
F=142

458.64

-4 1/2"

+4 1/2"

32.5

39

445

2+00

445

C=37
F=98

458.84

-2 1/2"

+1"

31

NOTE:
GRAVEL BASE VARIES FROM EXIST. DEPTH
AT STA. 1+00 TO 18" AT STA. 2+00

GRUBBING IN FILL
STA. 1+25 TO STA. 3+50 ±

445

1+50

445

C=23
F=62

459.04

LIMIT OF WORK
STA. 1+00

445

1+00

445

450

0+75

450

450

0+50

450

445

0+00

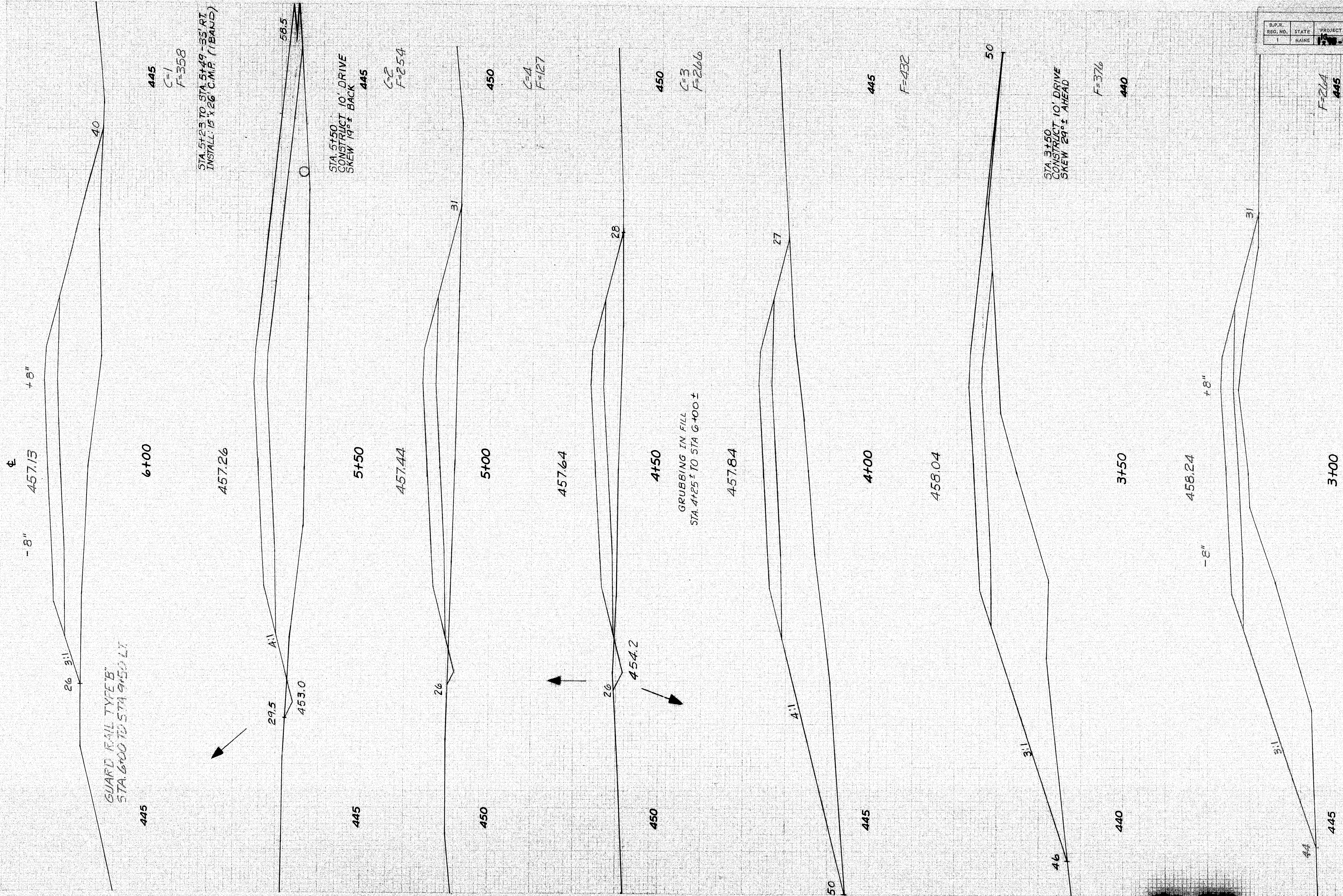
445

B.P.R.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	10-100	36	45



0+00 TO 2+50

S.H.G. & C.E.A. 1-65
 J.A.W. 2-65
 G.E.A. 3-65



B.P.R. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	1-1-0-0(43)	37	45

S.H.G. & C.E.A. 1-65
 J.A.W. 2-65
 G.E.A. 3-65